# FUNCTIONAL SERVICING REPORT 

Mixed Use Development 550 Ontario Street South, Milton

## 2613708 ONTARIO INC.



MANTECON PARTNERS INC.
STRUCTURAL \| MECHANICAL \| ELECTRICAL \| CIVIL ENGINEERING \& PROJECT MANAGEMENT

15 FOUNDRY STREET
DUNDAS, ON L9H 2V6

Project No. 19-109

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Appendix B - Sanitary Flow and Water Demand

### 1.0 Introduction

### 1.1 General Description and Criteria

## Introduction

Mantecon Partners Inc. has been retained by 2613708 Ontario Inc. to prepare a Functional Servicing Report (FSR) for the proposed mixed-use development being built at 550 Ontario Street South, in Milton, Ontario, with property limits as shown below in Figure 1. This FSR report is being prepared in support of the Zoning By-law Amendment Application and will demonstrate how servicing of the proposed development is achievable and to provide a basis for the Site Plan Application.

## Existing Site Conditions

The existing 550 Ontario Street South site footprint is approximately 1.68 ha, and currently contains restaurant and commercial/retail development, to be demolished. The site is bounded by residential use to the north with commercial use to the east and south across Ontario Street and Derry Road, with 16 Mile Creek block to the west of the proposed development.

A topographic survey was prepared by MacKay, MacKay \& Peters Limited dated February 12, 2020.
Refer to the Aerial Map below for the subject lands.


Figure 1 - Subject Site

## Proposed Development

The new proposed development area is 1.57 ha with the incorporated 4.1 m road widenings on both Derry Road and Ontario Street and includes the daylight triangle at the Ontario Street / Derry Road intersection.

The proposed mixed-use development is comprised of three (3) levels of underground parking and an above grade parking area comprising a total of 848 parking stalls including the required $3 \%$ of accessible stalls. The building heights vary to 24 stories. The development will contain approximately $2,039 \mathrm{~m}^{2}$ of commercial space and 649 residential suites.

The development consists of three (3) main building footprints with varying story heights and the limits of the underground parking encompasses the combined common limits of the 3 building footprints. At-grade parking is in the middle of the site and is accessed from Ontario Street. The upper level of underground parking is accessed from Derry Road as this entrance is roughly 2.75 m lower than the Ontario Street access. Refer to Figure 1 - Site Plan, below for orientation.

As the buildings and limit of underground parking garage covers approximately $76 \%$ of the proposed site area, the proposed development's stormwater management system will consist of collecting stormwater from building roofs and the interior parking area and storing the runoff within the underground parking area (upper level) and providing controlled release to the 16 Mile Creek system as is currently the existing condition in operation and allowing the perimeter areas to drain uncontrolled off-site accordingly. The required stormwater storage system will be within the building footprint as there is little room outside the building to accommodate an underground tank or superpipe system that will not be impacted by the required excavation for the deep building basement levels. Refer to the Stormwater Management Report under separate cover for further information.


Figure 1 - Site Plan

### 2.0 Water Supply and Distribution

### 2.1 Existing Watermain

There is an existing 400 mm diameter D.I watermain along the north side of Derry Road with two existing service locations to the 550 Ontario Street property. There are 200 mm diameter and 50 mm diameter services near the west side of the subject property and 150 mm diameter and 38 mm diameter services near the east side of the subject property along Derry Road. The west services support the main commercial/restaurant building and the east services support the standalone restaurant building.

Along the west side of Ontario Street exists an abandoned 200 mm D.I. diameter watermain that was replaced with a 300 mm diameter D.I. watermain.

One (1) fire hydrant exists on Derry Road near the west side of the subject property. It is to remain in place.
Two (2) fire hydrants exist on Ontario Street fronting the proposed development and are to remain in place.

### 2.2 Proposed Watermain

The existing 200 mm diameter fire service noted along the west side of Derry Road is to remain in use and is to be extended to the proposed building. The 50 mm diameter domestic water service is to be removed and replaced with a new 100 mm diameter domestic water service with new connection to the existing 400 mm watermain and extend to the proposed building. A meter and backflow preventor will be required immediately upon entry to the building for the domestic service and a backflow preventor is required for the 200 mm fire main.

The estimated water demand for the proposed development based on Region of Halton requirements is approximately $6.8 \mathrm{~L} / \mathrm{sec}$. Refer to Appendix B for calculations.

No new fire hydrants are proposed. The fire hydrant on Ontario Street, just north of the intersection, will be within 45 m of the Fire Department Connection at the main entrance of Building 1. The fire hydrant on Derry Road will also be within 45 m of a second Fire Department Connection on Building 2.
The proposed buildings will be sprinkled. Given the varied height of the building towers (+/-24 stories), internal building fire pumps will be required.
Two (2) fire hydrants adjacent to the proposed site have been tested by Jackson Waterworks and information is provided in Appendix A. The fire hydrants tested were located on Derry Road at about 20 m east of the west property line and on Ontario Street South at about 18m north of the Derry Road/Ontario Street intersection.

### 3.0 Sanitary Sewerage

### 3.1 Existing Sanitary Sewer

There is an existing 250 mm diameter A.C. sanitary sewer in Ontario Street South along the east side of the road flowing northerly to Laurier Avenue.
Sanitary sewage from the existing site outlets to an existing manhole and sewer at the northwest corner of the site. This sewer flows northerly via a 250 mm diameter A.C. sewer within an existing 4.57 m wide easement to Laurier Avenue. The existing sewer from the site building is a 150 mm diameter PVC lateral with slope of approximately $5.5 \%$. This existing 150 mm sewer will be removed and replaced with 200 mm diameter piping.

### 3.2 Proposed Sanitary Sewer

Sewage flow from the proposed building will utilize the existing 250 mm sanitary sewer at the northwest corner of the site, in a grassed/scrub area, as this is the most cost effective and least disruptive to off-site facilities. The alternative 250 mm sanitary sewer along the east side of Ontario Street is much more challenging due to the impact to the road and local traffic, requiring road restoration and traffic control.

The new 200 mm diameter PVC, SDR-35 sewer at $2 \%$ slope will be constructed from the existing sanitary MH in the easement to an inspection manhole near the property line (on the private side). From the proposed inspection manhole, a new 200 mm diameter PVC lateral at $2 \%$ slope will be constructed to the proposed building. It is intended to keep the sewer as deep as possible to drain lower levels of the building. The benching and connection in the existing sanitary manhole will need to be reworked to accommodate the new 200mm sewer, accordingly.

The estimated flow from the proposed development based on Halton Region requirements is approximately $6.61 \mathrm{~L} / \mathrm{sec}$. Refer to Appendix B for calculations.

### 4.0 Grading and Drainage

### 4.1 Existing Grading and Drainage

The existing site has an "L" shaped strip mall with stand alone restaurant in the parking lot. The extent of the site is mainly paved with curbing and site drainage that utilizes parking lot catchbasins and roofs to collect stormwater and convey it by underground piping to a manhole near the southwest corner of the site. This manhole has controlled stormwater release via orifice control to the adjacent 16 Mile Creek system.

Grading around the property varies by more than 4m. Along Ontario Street it is relatively flat, from 195.75 m at the north property line to 195.35 m at the Ontario St. / Derry Rd. intersection. Frontage along Derry Road varies from 195.00 m at the intersection to 191.60 m at the west property line. The west property line varies and rises up from the south property line of 191.60 m to 193.50 m and then back down 191.40 m at the north property line. The north property line then slopes easterly from 191.400 m back up to 195.75 m .

### 4.2 Proposed Grading and Drainage

The proposed mixed-use development has a building footprint and underground parking garage limit that covers approximately $76 \%$ of the site. These large roof areas and interior parking area will collect stormwater and convey it to an interior holding tank for controlled release to the adjacent 16 Mile Creek Block through the existing storm sewer outlet control manhole. For further information, refer to the Stormwater Management Report under separate cover.

The internal parking lot area will be graded to utilize area drains with internal piping to be designed by a mechanical engineer and conveyed to an oil grit separator and then to the storage tank. The major overland flow from the interior parking lot area will be directed to the curb opening at the rear of Building 3 and then conveyed by grassed swale to the 16 Mile Creek.

The proposed outlet control manhole near the southwest corner of the site is proposed to be constructed over the existing 300 mm diameter storm sewer pipe. A new inlet pipe will be constructed that connects the interior storage tank system to the control structure manhole. It is proposed to provide a stainless-steel orifice plate with orifice for controlled release to 16 Mile Creek and a CB beehive grate will be provided on this structure to allow for the potential blocked stormwater release or high-volume storm events beyond those designed for to escape out of the lid and be directed via a major overland flow route to the adjacent creek area.

The frontages along Derry Road and Ontario Street will have sheet flow drainage to the existing right-of way storm sewer system. These frontages will contain both hard and soft surface features.

The west and north limits of the site are mainly grassed and/or treed and will drain to the adjacent Creek Block via sheet flow from the west side of the site and swale flow from the north side as the rear grade drops more than 3 m from east to west.

### 5.0 Summary

Based on the information provided herein, servicing constraints should not be a concern during the consideration of the planning and development for this mixed-use development site. It is concluded that the proposed development at 550 Ontario Street South can be serviced with respect to Water Supply and Distribution, Sanitary Sewerage, and Grading and Drainage, to meet the requirements of Town of Milton and Halton Region.
We trust that the information provided is satisfactory. Should you have any questions please do not hesitate to contact our office at (905) 648-0373.

Respectfully submitted,
Mantecon Partners Inc.,


Per: Michael Dessureault, P. Eng. Senior Project Manager, Civil
MD/jm


# 550 ONTARIO STREET SOUTH <br> TOWN OF MILTON, ON 

## MIXED USE APARTMENT DEVELOPMENT FOR 2613708 ONTARIO INC. <br> JOB NO. 19-109

## ISSUED FOR ZONING APPLICATION



KEY PLAN

## INDEX TO DRAW/NGS

SHEET No. DESCRIPTION
CIVIL
Cooo
C000
C001
C100
C200
C 300
C300
C400
cover sheet
Existing conditions plan
site servicing plan
SITE GRADING PLAN
SIte sediment and erosion control plan
site storm drainage area plan






APPENDIX A

## RATED THEORETICAL CAPACITY OF FIRE HYDRANTS

Project Name: 550 Ontario Street
Project Number: 19-109
Date: July 2020
Prepared by: M. Dessureault

CALCULATION TO DETERMINE THE PREDICTED FLOW OF A FIRE HYDRANT PER NFPA GUIDELINES AND BASED ON THE RATED THEORETICAL CAPACITY AT 20 PSI

Test performed by Jackson Waterworks on March 20, 2020

## Hydrant Flow Test Results \#1 (Derry Road)

| Predicted Flow (PF) | $=$ | 20 | (Predicted Flow is always 20 psi ) |
| :--- | :--- | :---: | :--- |
| Static Pressure (SP) | $=$ | 92 | (per hydrant flow test results) |
| Residual Pressure (RP) | $=$ | 88 | (per hydrant flow test results) |
| Flow (USGPM) | $=$ | 1278 | (Residual Flow per hydrant flow test results-Imperial Units) |

## Theoretical Fire Flow Calculation @ 20 psi

| (1) $=$ SP - PF | = | 72 |  |
| :---: | :---: | :---: | :---: |
| (2) $=$ SP - RP | = | 4 |  |
| (3) $=(1) /(2)$ | = | 18.000 |  |
| (4) $=(3)^{\wedge} 0.54$ | = | 4.763 |  |
| (5) = Flow x (4) | = | 6087 | USGPM |

## USGPM to L/min Conversion

| Flow (USGPM) | $=$ | 6087 |
| ---: | :--- | :---: |
| Flow (UKGPM) | $=$ | 5068 |
| Flow (L/sec) | $=$ | 384 |

## Hydrant Flow Test Results \#2 (Ontario Street)

| Predicted Flow (PF) | $=$ | 20 | (Predicted Flow is always 20 psi ) |
| :--- | :--- | :--- | :--- |
| Static Pressure (SP) | $=$ | 87 | (per hydrant flow test results) |
| Residual Pressure (RP) | $=$ | 81 | (per hydrant flow test results) |
| Flow (USGPM) | $=$ | 1278 | (Residual Flow per hydrant flow test results-Imperial Units) |

## Theoretical Fire Flow Calculation @ 20 psi

| $(1)=S P-P F$ | $=$ | 67 |
| :--- | :--- | :--- |
| $(2)=S P-R P$ | $=$ | 6 |
| $(3)=(1) /(2)$ | $=$ | 11.167 |
| $(4)=(3)^{\wedge} 0.54$ | $=$ | 3.680 |
| $(5)=$ Flow $\times(4)$ | $=$ | 4703 USGPM |

## USGPM to L/min Conversion

| Flow (USGPM) | $=$ | 4703 |
| ---: | :--- | :---: |
| Flow (UKGPM) | $=$ | 3916 |
| Flow (L/sec) | $=$ | 297 |

Mr. AngeloCutaia
AC III Group
3380 South Service Road
Burlington Ontario L7N 3 J5
08 April 2020

Jackson Waterworks has recently completed fire hydrant flow testing at 550 Ontario Street South in Milton.
We define the Test Hydrants as the ones being flowed, and the Base Hydrant as the one where static and residual pressures are recorded. Wherever possible, we inspect the secondary valve for the Test Hydrants to make sure it is in the fully open position. Likewise, we count the number of turns needed to open the Test Hydrants (to make sure it is opening completely).

We do not use pitot conversion factors for different nozzle profiles. The Engineer may use these factors if desired and warranted.

The secondary valve for the Test Hydrant was found to be fully open at the time of test \#2. It could not be inspected at the time of test \#1.

Testing was completed in accordance with NFPA 291 guidelines.
There were no irregularities to report.

Trusting this meets with your approval, we are...
Yours truly,


Mark Schmidt Jackson Waterworks


| \# of Ports | PORT DIA. (in/mm) | PITOT (psig) | FLOW (usgpm) | RESIDUAL (psig) |
| :---: | :---: | :---: | :---: | :---: |
| 1 | $2.50 / 63$ | 58 | 1278 | 88 |
| 2 | $2.50 / 63$ | $44 / 44$ | 2226 | 83 |
| THEORETICAL FLOW @ 20psi |  |  |  |  |


| General Data |  |
| :---: | :---: |
| Test Date | 20 March 2020 |
| Test Time | $09: 30 \mathrm{am}$ |
| Pipe Dia. | $16 "$ |
| Static | 92 |

Site Information

| Site or Developer Name | AC III Group |
| :--- | :--- |
| Site Address/Municipality | 550 Ontario Street South, Milton |
| Location of Test Hydrant | Derry Road, 1st West of Ontario Street South |
| Location of Base Hydrant | Ontario Street South, 1st North of Derry Road |
|  | No conversion factor used for flow calculation based on round and flush internal nozzle |
|  | configuration. Flow testing has been conducted in accrodance with NFPA 291 guidelines |
|  | wherever possible. Refer to attached report for further information. |
|  | Verified By: Mark Schmidt |



| \# of Ports | PORT DIA. (in/mm) | PITOT (psig) | FLOW (usgpm) | RESIDUAL (psig) |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 2.50/63 | 58 | 1278 | 81 |
| 2 | 2.50/63 | 42/42 | 2174 | 73 |
| THEORETICAL FLOW @ 20psi |  |  | 4703 |  |


| General Data |  |
| :---: | :---: |
| Test Date | 20 March 2020 |
| Test Time | $09: 00 \mathrm{am}$ |
| Pipe Dia. | $12 "$ |
| Static | 87 |


| Site Information |  |
| :--- | :--- |
| Site or Developer Name | AC III Group |
| Site Address/Municipality | 550 Ontario Street South, Milton |
| Location of Test Hydrant | Ontario Street South, 2nd North of Derry Road |
| Location of Base Hydrant | Ontario Street South, 1st North of Derry Road |
| Technician's Comments | No conversion factor used for flow calculation based on round and flush internal nozzle |
|  | configuration. Flow testing has been conducted in accrodance with NFPA 291 guidelines |
|  | wherever possible. Refer to attached report for further information. |
|  | Verified By: Mark Schmidt |

The Regional Municipality of Halton
1151 Bronte Road
Oakville ON L6M 3L1
Dear Sir/Madam:

## RE: Water Usage and Sanitary Discharge Report for Proposed Mixed-Use Development, 550 Ontario Street South, Town of Milton

## Background

The existing strip mall development at 550 Ontario Street South in the Town of Milton is proposed to be demolished and a mixed-used development with towers to 24 storeys high constructed containing mostly residential use with some at-grade commercial use and underground parking.

1. Commercial use (at-grade) $-2,039 \mathrm{~m}^{2}$;
2. Residential use (apartments) - 649 units (408 1-bedroom \& 241 2-bedroom).

The site is currently an existing strip mall that will be demolished.
The site has an area of $\pm 1.57$ ha of which approximately $3,885 \mathrm{~m}^{2}$ will be landscaped.
Table 8.2.1.3.A \& B of the Ontario Building Code has been used to calculate water usage and sanitary discharge for occupant loadings. This mixed-use development does not require water in the process and cooling water and it will not be required.

## Sanitary Discharge

Assume Restaurant use (as a worst-case scenario) (not 24 hr at 125 L per seat per day):

1. Restaurant Space $=2,039 \mathrm{~m}^{2}$

Use $80 \%$ area for seating and 4 seats per $10 \mathrm{~m}^{2}=(2,039 \times 0.80) / 10 \times 4=652$ seats (estimated)

$$
\begin{aligned}
& 652 \times 125 \mathrm{~L}=81,500 \mathrm{~L} / \text { day } \\
& =81.5 \mathrm{~m}^{3} / \text { day } \\
& =\underline{0.94 \mathrm{~L} / \mathrm{sec}}
\end{aligned}
$$

Residential use (Table 8.2.1.3.A Residential Occupancy and Subsection 3.1.17):

1. 649 units (Apartments, Condominiums at $275 \mathrm{~L} / \mathrm{ca} /$ day)
a. 4081-bedroom
b. 241 2-bedroom
$408 \times 2$ people per bedroom $=816$ people $241 \times 4$ people per 2 -bedroom $=964$ people Total people $=1,780$ people

Total Flow $=1,780 \times 275 \mathrm{~L} /$ day $=489,500 \mathrm{~L} /$ day
$=489.5 \mathrm{~m}^{3} /$ day
$=\underline{5.67 \mathrm{~L} / \mathrm{sec}}$

> Total sanitary discharge from site $=$ commercial use + residential use $=0.94 \mathrm{~L} / \mathrm{sec}+5.67 \mathrm{~L} / \mathrm{sec}$ $=6.61 \mathrm{~L} / \mathrm{sec}$ or $\left(572 \mathrm{~m}^{3} / \mathrm{day}\right)$

## Water Usage

## Mixed-Use building:

Total water usage from proposed mixed-use development $=\underline{\mathbf{5 7 2} \mathrm{m}^{3} / \text { day }}$ (see above) $=6.61 \mathrm{~L} / \mathrm{sec}$

No process water usage.
No cooling water usage.

## Landscaping for site:

-Site landscaping area $=3,885 \mathrm{~m}^{2}$
$25.4 \mathrm{~mm} / \mathrm{m}^{2} /$ week $\times 3,885 \mathrm{~m}^{2} / 7 / 1000=\underline{14.1 \mathrm{~m}^{3} / \text { day }}$
Total landscape water usage from site $=14.1 \mathrm{~m}^{3} /$ day
Total water usage from site $=572 \mathrm{~m}^{3} /$ day $+14.1 \mathrm{~m}^{3} /$ day $=586 \mathrm{~m}^{3} /$ day $=6.8 \mathrm{~L} / \mathrm{sec}$
Sincerely,

## MANTECON PARTNERS INC.

STRUCTURAL, MECHANICAL, ELECTRICAL, CIVIL ENGINEERING AND PROJECT MANAGEMENT


Michael Dessureault, P.Eng.
Senior Project Manager / Civil Engineer
MD/jm

