

Consulting Engineers and Scientists

Geotechnical Investigation Proposed Commercial Warehouse Development

6728 Sixth Line, Milton, Ontario

Submitted to:

Anatolia Investments Corporation 8300 Huntington Road Vaughan, ON L4H 4Z6

Submitted by:

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1. Introduction

GEI Consultants Ltd. (GEI) was retained by Anatolia Investments Corporation (the Client) to complete a geotechnical investigation and report for the Proposed Commercial Warehouse Developments at 6728 Sixth Line in Milton, ON. A site location plan is enclosed as Figure 1.

The site is located at 6728 Sixth Line in Milton, Ontario, and comprises the land in the southwest quadrant of the Derry Road West and Sixth Line Intersection. The site is about 625 m north/south and about 1,000 m east/west. The site is currently the location of the former Trafalgar Golf and Country Club and has a few light structures associated with the golf facility, with predominant landscaped golf course areas. It is understood that some of the structures have been demolished in preparation for the redevelopment.

It is understood that three commercial buildings are proposed, and a grade raise is proposed for most of the site. Building 1 will be about 240 m by 450 m in plan (proposed slab Elev. 191.90), Building 2 will be about 180 m by 330 m in plan (proposed slab Elev. 192.10) and Building 3 will be approximately 85 m by 225 m in plan (proposed slab Elev. 191.05). All three buildings will be slab-on-grade and will have some form of truck loading docks on portions of the buildings. Paved parking and access will surround the buildings. Three Storm Water Management (SWM) facilities are proposed, one underground south of Building 1 and two at grade ponds south of Buildings 2 and 3 respectively. A new road (Clark Boulevard) is proposed between Buildings 1 and 2, connecting to Derry Road West in the north. A future watercourse channel is proposed between Buildings 2 and 3 and along the south of Buildings 1 and 2 (a re-alignment of the existing watercourses). An aerial image of the site is provided on Figure 2A and the proposed concept plan is included as Figure 2B.

The purpose of the geotechnical investigation was to assess the subsurface soil conditions at the site, and based on this information, provide geotechnical engineering recommendations in support of the proposed development. This report summarizes the borehole findings, provides design geotechnical engineering recommendations regarding available bearing capacities for foundations, slabs-on-grade, site servicing installation, SWM pond/infiltration and pavement design. Considerations for constructability such as soil excavation, compaction, on-site backfill suitability and temporary groundwater control are also provided.

It is noted that the recommendations provided in this report must be considered preliminary in nature due to the current uncertainty of the design for the project. As the design progresses further geotechnical review and input may be required which might necessitate the need for additional investigation and/or analysis.

GEI has also been retained to complete a hydrogeological study and slope stability assessment for the site and the findings and recommendations are provided under separate covers.



2. Procedures and Methodology

It is noted that all elevations in this report are metric/geodetic and expressed in metres (m). All measurements are also in metric and expressed in millimetres (mm), metres (m) or kilometres (km).

Prior to the commencement of drilling activities, the borehole locations were staked in the field by GEI. Ground surface elevations of the boreholes and horizontal co-ordinates (referencing NAD 83 geodetic datum) were surveyed by GEI with a Topcon FC – 5000 GPS Survey unit.

Underground utilities including natural gas, electrical, telephone, water, etc. were marked out by public and private utility locating companies prior to drilling.

The fieldwork for the drilling program was carried out between April 21 and May 1, 2023. Boreholes 1 to 42, 44 and 48 to 54 were advanced from 5.0 to 8.1 m below existing grade (Elev. 179.1 to 187.4). Boreholes 43, 45, 46 and 47 were drilled earlier for the slope stability assessment on March 6, 2023, and were advanced to 5.0 to 8.1 m depth (Elev. 180.2 to 183.5). Borehole logs are provided in Appendix A and the borehole locations are shown on Figure 2A (aerial image) and Figure 2B (concept plan).

The boreholes were advanced by a drilling subcontractor retained and supervised by GEI using a track-mounted drill rig, solid stem augers, and standard soil sampling equipment. Sampling was conducted using a 51 mm O.D. Split Spoon (SS) sampler. Standard Penetration Test (SPT) "N" Values (N values) were recorded for the sampled intervals as the number of blows required to drive an SS sampler 305 mm into the soil using a 63.5 kg drop hammer falling 750 mm, in accordance with ASTM D1586. In each borehole soil sampling was conducted at 0.75 m intervals for the upper 3.0 m and at 1.5 m intervals thereafter.

Monitoring wells were installed in Boreholes 1, 2, 13, 14, 22 to 24, 26, 28, 31, 35, 37, 40, 43, 45 and 52 by GEI to facilitate long-term groundwater monitoring, each consisting of 50 mm diameter PVC pipe with a 1.5 m long screen and protective casing. Monitoring well construction is shown on the borehole logs in Appendix A. Boreholes without wells were backfilled in accordance with O.Reg. 903.

The GEI field staff examined, and classified characteristics of the soils encountered in the boreholes, including the presence of fill materials, groundwater observations during and upon completion of the drilling, recorded observations of borehole construction, and processed the recovered samples. All recovered soil samples were logged in the field, carefully packaged, and transported to GEI's laboratory for more detailed examination and classification.



In GEI's laboratory, the samples were classified as to their visual and textural characteristics. A total of nineteen (19) representative samples of the major soil units were selected and submitted to our laboratory for grain size analysis. Seven (7) of the samples were also submitted for Atterberg Limits tests. Laboratory results are provided in Appendix B. It is noted that the laboratory testing from the slope stability report is presented on separate figures.



3. Subsurface Conditions

3.1 General Overview

The detailed soil profiles encountered in the boreholes are indicated on the attached borehole logs in Appendix A, and the geotechnical laboratory results are included in Appendix B. The borehole locations are shown in Figures 2A and 2B.

It should be noted that the conditions indicated on the borehole logs are for specific locations only and can vary between and beyond the locations. It should be noted that the soil boundaries indicated on the borehole logs are inferred from non-continuous sampling and observations during drilling. These boundaries are intended to reflect approximate transition zones and should not be interpreted as exact planes of geological change.

In addition, the descriptions provided in the borehole logs are inferred from a variety of factors, including: visual observations of the soil samples retrieved, laboratory testing, measurements prior to and after drilling, and the drilling process itself (speed of drilling, shaking/grinding of the augers, etc.). The passage of time also may result in changes in conditions interpreted to exist at locations where sampling was conducted.

3.2 Stratigraphy

3.2.1 Topsoil

A surficial topsoil layer was at the ground surface in all boreholes except Boreholes 17 and 27 ranging in thickness from 50 to 280 mm. In Borehole 27, the 180 mm topsoil layer was buried beneath the surficial fill.

3.2.2 Fill

A fill layer was encountered in most boreholes and considered to be associated with final grading for the golf course. Boreholes 3 to 8, 10 to 13, 15, 17 to 21, 23 to 34, 36, 37, 40, 41, and 43 to 54 encountered fill below the topsoil, locally at the surface, and the fill was penetrated at 0.2 to 2.3 m depth (Elev. 185.0 to 191.6). The fill predominantly consisted of clayey silt or sandy clayey silt or sandy silt, varying to sand and silt or silty sand. Trace organics and rootlets were observed in some boreholes. The fill was moist to wet with moisture contents of 10 to 32%. The fill typically had N values of 10 or less revealing soft to stiff / very loose to compact conditions.



3.2.3 Clayey Silt / Sandy Clayey Silt / Clay and Silt

A cohesive unit of soil comprising clayey silt, sandy clayey silt, or clay and silt was encountered below the fill and/or the topsoil, locally the discontinuous cohesionless soil layers (described below), in most boreholes, except Boreholes 16, 18, 34, 43, 49, 50- 53 and 54. The unit was penetrated at depths of 1.1 to 4.6 m (Elev. 183.0 to 189.9). Grain size analysis results of seven (7) samples are included in Figures B1 and B3 in Appendix B. Atterberg limits tests on three samples are provided in Figure B2, with plastic limits of 15.7 to 18.7 and liquid limits of 27.5 to 35.9. The moisture contents ranged between 8 and 28%, being moist to wet. The N values in these layers ranged between 4 to more than 50 blows, indicating firm to hard conditions but typically stiff to very stiff.

3.2.4 Sandy Silt / Silt / Sand and Silt / Sand / Sand and Gravel

Localized layers of cohesionless soil consisting of sandy silt, silt, sand and silt, sand or sand and gravel were observed below the upper clayey soil layers and/or the topsoil in Boreholes 1, 11, 16, 35, 38, 42 and 53. These layers were observed at depths of 0.2 to 2.6 m (Elev. 187.1 to 189.8) and extended to 1.5 to 3.5 m depth (Elev. 186.4 to 189.2), locally from 4.6 to 4.7 m depth (Elev. 186.4 to 186.5) in Borehole 38 and 4.6 to 5.0 m depth (Elev. 182.6 to 186.2) in Borehole 27. One (1) sample of the sand material was submitted for grain size analysis and the results are provided in Figure B4 in Appendix B. The soil was moist to wet with moisture contents of 16 to 25%. The soil was loose to compact, and N values ranged from 5 to 29 blows.

3.2.5 Glacial Till

A glacial till deposit was encountered in all boreholes below the upper soil layers and extended below the 5.0 to 8.1 m depth of exploration (Elev. 179.1 to 187.4). The till matrix predominantly consisted of sandy silt or sandy clayey silt, locally grading to clayey silt or gravelly sand. Cobbles and boulders should be expected based on augers grinding during advancement of the boreholes. Eleven (11) samples of the material were submitted for grain size analysis and the results are provided in Figures B5 and B6 in Appendix B. Four (4) samples were also submitted for Atterberg Limit tests and the results are presented on Figure B7 in Appendix B. Plastic limits of 8.0 to 16.9 and liquid limits of 20.4 to 34.0 were revealed from the lab testing. The glacial till was brown, brownish grey or grey near the base, and was typically moist to wet, with moisture contents ranging from 6 to 29%, typically 7 to 12%. N values were 4 to more than 50 blows, indicating compact to very dense / firm to hard conditions, typically compact / stiff to very stiff.



3.3 Groundwater

Unstabilized groundwater level measurements and cave measurements were taken upon the completion of drilling of each borehole as shown on the borehole logs in Appendix A. These measurements were taken to provide a rough estimate of the possible excavation and temporary groundwater control constructability considerations that may arise. Sixteen (16) boreholes were outfitted with a monitoring well with 50 mm diameter PVC standpipe and 1.5 m long screen. Monitoring well configuration and groundwater observations are noted on the borehole logs in Appendix A, and a summary is below.

Borehole	Depth of Cave (m) / Elev.	Unstabilized Groundwater Level Depth / Elev.	Depth / Elev. of Groundwater Table, May 23, 2023
1	Open (8.1 / 184.2)	No Water	0.7 / 191.5
2	Open (5.0 / 186.4)	No Water	0.5 / 190.7
3	Open (8.1 / 184.0)	7.6 / 184.5	N/A
4	Open (5.0 / 186.6)	No Water	N/A
5	3.9 / 186.7	3.0 / 187.6	N/A
6	Open (8.1 / 183.3)	7.6 / 183.8	N/A
7	Open (5.0 / 185.5)	3.2 / 187.1	N/A
8	Open (8.1 / 182.4)	7.4 / 183.1	N/A
9	Open (5.0 / 185.4)	No Water	N/A
10	Open (5.0 / 185.1)	No Water	N/A
11	Open (8.1 / 183.8)	7.3 / 184.6	N/A
12	Open (5.0 / 184.9)	No Water	N/A
13	Open (5.0 / 185.3)	No Water	1.0 / 189.3
14	Open (5.0 / 185.1)	No Water	0.5 / 189.5
15	Open (5.0 / 184.3)	No Water	N/A
16	Open (5.0 / 184.6)	No Water	N/A
17	Open (5.0 / 184.9)	No Water	N/A
18	6.7 / 184.1	4.5 / 186.3	N/A
19	Open (5.0 / 184.7)	4.5 / 185.1	N/A
20	Open (5.0 / 184.5)	4.1 / 185.4	N/A
21	Open (8.1 / 181.0)	5.7 / 183.4	N/A
22	Open (5.0 / 184.4)	No Water	0.4 / 189.0
23	Open (8.1 / 181.2)	6.0 / 183.3	0.9 / 188.4



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Borehole	Depth of Cave (m) / Elev.	Unstabilized Groundwater Level Depth / Elev.	Depth / Elev. of Groundwater Table, May 23, 2023
24	4.8 / 184.7	4.5 / 185.0	0.6 / 188.9
25	3.6 / 186.0	1.8 / 187.8	N/A
26	Open (5.0 / 184.3)	No Water	1.1 / 188.2
27	Open (5.0 / 182.3)	No Water	N/A
28	Open (5.0 / 184.5)	1.5 / 188.0	0.9 / 188.6
29	Open (8.1 / 182.4)	No Water	N/A
30	4.8 / 187.0	4.4 / 187.4	N/A
31	Open (8.1 / 182.4)	No Water	1.0 / 189.5
32	Open (8.1 / 182.9)	7.0 / 184.0	N/A
33	7.0 / 183.6	4.6 / 186.0	N/A
34	Open (8.1 / 179.1)	7.0 / 180.2	N/A
35	Open (5.0 / 186.4)	No Water	0.4 / 191.0
36	Open (5.0 / 187.4)	2.1 / 190.3	N/A
37	Open (8.1 / 183.9)	7.1 / 184.9	1.2 / 190.8
38	Open (5.0 / 186.2)	4.2 / 187.0	N/A
39	Open (5.0 / 185.5)	No Water	N/A
40	Open (5.0 / 185.2)	4.2 / 186.0	0.5 / 189.7
41	Open (5.0 / 184.8)	No Water	N/A
42	Open (5.0 / 183.7)	4.5 / 184.1	N/A
43	Open (5.0 / 180.8)	No Water	0.0 / 185.8 (Mar 30, 3023)
44	Open (5.0 / 181.1)	No Water	N/A
45	Open (5.0 / 183.5)	4.6 / 183.9	2.7 / 185.8 (Mar 30, 3023)
46	Open (5.0 / 181.5)	No Water	N/A
47	Open (8.1 / 180.2)	6.7 / 181.6	N/A
48	Open (5.0 / 184.6)	No Water	N/A
49	Open (5.0 / 183.4)	No Water	N/A
50	4.8 / 182.6	4.5 / 182.9	N/A
51	3.6 / 185.1	0.9 / 187.8	N/A
52	5.6 / 184.3	4.7 / 184.2	Well Destroyed
53	3.3 / 186.3	0.9 / 188.7	N/A



Borehole	Depth of Cave (m) / Elev.	Unstabilized Groundwater Level Depth / Elev.	Depth / Elev. of Groundwater Table, May 23, 2023
54	0.6 / 187.3	0.9 / 187.0	N/A

The stabilized groundwater levels in the monitoring wells are typically within about 1 m of the ground surface corresponding to Elev. 185.8 to 191.5. This is believed to reflect perched water in the fill above the clayey silt/sandy clayey silt/clay and silt and underlying till.

The materials at the site are generally not permeable. Localized wet seams and permeable soil layers are present.

Groundwater levels are expected to show seasonal fluctuations and vary in response to prevailing climate conditions.



4. Engineering Design Parameters & Analysis

The site is located at 6728 Sixth Line in Milton, Ontario, and comprises the land in the southwest quadrant of the Derry Road West and Sixth Line Intersection. The site is about 625 m north/south and about 1,000 m east/west. The site is currently the location of the former Trafalgar Golf and Country Club and has a few light structures associated with the golf facility, with predominant landscaped golf course areas. It is understood that some of the structures have been demolished in preparation for the redevelopment.

It is understood that three commercial buildings are proposed, and a grade raise is proposed for most of the site. Building 1 will be about 240 m by 450 m in plan (proposed slab Elev. 191.90), Building 2 will be about 180 m by 330 m in plan (proposed slab Elev. 192.10) and Building 3 will be approximately 85 m by 225 m in plan (proposed slab Elev. 191.05). All three buildings will be slab-on-grade and will have some form of truck loading docks on portions of the buildings. Paved parking and access will surround the buildings. Three Storm Water Management (SWM) facilities are proposed, one underground south of Building 1 and two at grade ponds south of Buildings 2 and 3 respectively. A new road (Clark Boulevard) is proposed between Buildings 1 and 2, connecting to Derry Road West in the north. A future watercourse channel is proposed between Buildings 2 and 3 and along the south of Buildings 1 and 2 (a re-alignment of the existing watercourses). An aerial image of the site is provided on Figure 2A and the proposed concept plan is included as Figure 2B.

It is noted that the recommendations provided in this report must be considered preliminary in nature due to the current uncertainty of the design for the project. As the design progresses further geotechnical review and input may be required which might necessitate the need for additional investigation and/or analysis.

4.1 Site Grading

The latest concept drawings for the project were provided by the Client. Grading plans were not a part of the package, however Building 1 will have the ground floor set at Elev. 191.90, Building 2 will have the ground floor set at Elev. 192.10, and the Building 3 floor slab will be set at Elev. 191.05. In all cases the floor slab is at or just above the existing ground surface. When grading is established, GEI should review the drawings for geotechnical requirements.

The topsoil, fill and localized areas of weak native soil directly below the fill/topsoil are unsuitable to support the building. In this regard, it is recommended to strip the topsoil and stockpile separately then sub-excavate the fill (0.2 to 2.3 m depth) and/or upper weak native soil (typically about 0.8 m below grade where fill is not present, locally down to 1.5 m depth). The exposed competent native soil should be thoroughly compacted and then engineered fill



placement can commence, to the desired grade. Footings would be supported by native soil and/or the engineered fill and the floor slabs by the engineered fill.

4.1.1 Engineered Fill

GEI defines "engineered fill" as material that will support foundations, and which is placed and compacted in a specified and controlled manner under full-time supervision of geotechnical engineering staff.

In any location where engineered fill will be placed to raise grades or replace poor/weak soil, the topsoil, vegetation, weathered/disturbed or existing earth fill must be fully removed down to competent native soil. As noted above, the weak native soil to 0.8 m depth (locally 1.5 m depth) must also be removed. The exposed subgrade soil must be proof-rolled and inspected by the geotechnical engineer to ensure all unsuitable material (e.g., organics, weak or soft soil, weathered / disturbed soil, deleterious materials, existing fill) is removed from the engineered fill footprint. Any unsuitable areas must be further sub-excavated and replaced with fill compacted to targeted 100% Standard Proctor Maximum Dry Density (SPmdd), minimum 98% SPmdd in building areas and 95% SPmdd in road and servicing areas.

Once the subgrade is approved, engineered fill can be placed. Engineered fill must be placed under the full-time supervision of a geotechnical engineer as required in the Ontario Building Code. The engineered fill may consist of excavated on-site inorganic cohesionless soils provided they have been moisture conditioned to a moisture content within 2% of optimum moisture content and do not contain organics, topsoil or deleterious material. It is recommended that any imported soil consist of Granular B (OPSS.MUNI 1010) and be first used in building areas, with suitable on-site soil used in landscaped or road areas. Engineered fill must be placed in loose lifts of 200 mm or less and compacted as noted above.

In wet subgrade areas, the first lift of engineered fill shall consist of 400 mm of Granular B Type II (OPSS.MUNI 1010). This will help to bridge the weaker subgrade and improve the ability to achieve the compaction specifications for subsequent engineered fill lifts.

The engineered fill must extend a minimum of 1 m out from all sides of the foundations and extend at a 1 horizontal to 1 vertical slope (1H:1V) down to the exposed subgrade. A typical detail for engineered fill pad dimensioning is included in Appendix C.



4.2 Foundation Design

4.2.1 Foundations on Native Soil

Based on the building slab elevations, exterior footings are assumed at 1.5 m depth below the top of slab elevations, locally deeper near in loading dock areas, and interior footings are assumed to be established about 0.7 m below the top of slab elevations.

Based on the discussion in Section 4.1, foundations cannot be supported on the upper 0.8 m, locally 1.5 m of soil near the surface (where fill is absent). Footings will need to extend to more competent native soil or be founded on engineered fill (discussed in the next section).

Based on the N values in the boreholes, it is recommended that foundations supported by native soil be design based on a geotechnical bearing resistance of 150 kPa at Serviceability Limit State (SLS) and a factored bearing resistance at Ultimate Limit State (ULS) of 225 kPa. The geotechnical reaction at SLS allows for 25 mm or less of total settlement.

It is noted that higher bearing resistances are available at depth in the till in most areas and can be provided if requested.

4.2.2 Foundations on Engineered Fill

If the foundations are supported on an engineered fill pad, constructed as discussed in Section 4.1.1, the spread or strip footings can be designed using the underlying native soil bearing capacity shown above, up to a maximum of 150 kPa at SLS and 225 kPa at ULS.

It is recommended that nominal reinforcing steel for stiffening of the foundation walls made on engineered fill be provided to help mitigate minor cracking due to minor differential settlement. The reinforcing steel in the poured concrete foundation walls may consist of 2-15M bars continuous at the top of the foundation wall, and 2-15M bars continuous at the bottom of the foundation walls. Typically, these bars are placed 100 to 200 mm from the top or bottom of the foundation wall, respectively. The reinforcing steel should extend a minimum of 3 m past any transition zones between engineered fill and native soil. A typical reinforcing steel detail for foundation walls placed on engineered fill is provided within Appendix C. The recommended nominal reinforcing steel should not be considered a structural design. The need for different or additional reinforcement should be reviewed by a structural engineer to ensure the original structural design intent of the structure is maintained.

4.2.3 General Foundation Considerations

All footings exposed to ambient air temperature throughout the year must be provided with a minimum of 1.2 m of earth cover or equivalent insulation for frost protection (25 mm of polystyrene insulation is equivalent to 300 mm of soil cover). The minimum strip and spread



footing widths to be used shall be dictated as per the Ontario Building Code, regardless of loading considerations. Footings stepped from one level to another must be at a slope not exceeding 7V:10H. Loading dock areas may require insulation on the inside of the foundation wall to protect the interior under slab fill from frost issues.

The foundation design parameters provided above are predicated on the assumption that the foundation subgrade surface is undisturbed, and that all earth fill, deleterious, softened, disturbed, organic, and caved material is removed. The foundation excavation must be done in such a way that groundwater is controlled to prevent any disturbance to the foundation base. The groundwater table must be lowered at least 1 m below the founding elevation prior to excavation to prevent disturbance to the foundation subgrade from groundwater seepage.

The foundation subgrade must be reviewed prior to concrete placement to ensure the foundation design parameters provided are applicable, and to provide remedial recommendations if necessary. If the foundation excavation will be open for a prolonged period of time, the foundation subgrade should be protected with a skim coat of lean mix concrete (applied immediately after inspection by the geotechnical engineer), to ensure that no deterioration will occur due to weather effects.

4.3 Seismic Site Classification

Section 4.1.8.4 of the Ontario Building Code provides values of the acceleration and velocitybased site coefficients (F_a and F_v) for various time periods, associated with specific Site Classes. These Site Classes are based on the energy-corrected Average Standard Penetration Resistance values and undrained shear strength within the upper 30 m of soil underlying the grade beams or foundations of the proposed structure. As the boreholes were advanced less than this depth at the site, the site classification recommendation provided below assumes that the soil conditions are similar below the drilled depth.

Underneath the proposed foundations, the subsoil will consist of generally compact to dense cohesionless soils. The Site Classification for Seismic Site Response is "D" for this site.



4.4 Earth Pressure Design Parameters

The loading dock area, underground pond or other retaining type walls must be designed to resist unbalanced lateral earth pressures imparted from the weight of adjacent soils. Lateral earth pressures are calculated using the following equation:

$$P = K[\gamma h + q]$$

where, P = the horizontal pressure at depth, h (m) K = the earth pressure coefficient (dimensionless) h = depth below ground surface (m) $\gamma = the bulk unit weight of soil, (kN/m³)$ q = surcharge loading (kPa)

The above equation assumes that a drainage system is present which prevents the build-up of any hydrostatic pressure behind the structure subjected to the unbalanced lateral earth pressures. If this is not the case, the equation must be revised to also incorporate the submerged unit weight of the soil multiplied by the earth pressure coefficient, in addition to the water pressure itself.

The values for use in the design of structures subjected to unbalanced lateral earth pressures at this site are as follows.

Call Trues	γ - Bulk Unit	φ - Friction	Earth Press	ure Coefficient (limensionless)
Soil Type	Weight (kN/m ³)	Angle (degrees)	Ka - Active	Ko – At-Rest	K _p - Passive
Compact Cohesionless Native Soil/Stiff Cohesive Native Soil/Engineered Fill	20.0	30	0.33	0.50	3.0

The calculation of the earth pressure coefficients is based on Rankine theory, which provides a conservative estimate as no friction between the soil and the structure is accounted for. The earth pressure coefficients provided above are applicable for flat ground surfaces beyond the structure and must be revised for sloping ground surfaces.

The earth pressure coefficients referenced within the above table are a function of the friction angle of the adjacent soil, and both the degree and direction of movement of the structure subjected to unbalanced lateral earth pressures. For structures that are restrained at the top (such as basement walls), the at-rest earth pressure coefficient will apply. For structures that allow for 0.1 to 1% of movement away from the soil (such as unrestrained retaining walls), the full active earth pressure coefficient will apply. For structures that allow for 1 to 10% of movement into the soil, the full passive earth pressure coefficient will apply. The percentage movement is based on the height of the structure.



Other types of structures such as shoring walls with multiple rows of tiebacks and soil nail walls are subject to different loading conditions and must be analyzed separately.

4.5 Floor Slabs

As discussed in Section 4.1. slab-on-grade support is envisioned to comprise engineered fill which is considered suitable.

The engineered fill can be disturbed by the construction operation. The subgrade for the slabon-grade must be proof-rolled and inspected by the geotechnical engineer, prior to the placement of an aggregate base. If any soft or weak subgrade areas are identified, or if there are areas containing excessive amounts of deleterious/organic material, they must be locally sub-excavated and backfilled with approved clean earth fill or imported granular material and compacted to a minimum of 98% SPmdd. The modulus of subgrade reaction appropriate for design of a slab-on-grade on approved engineered fill compacted fill, or undisturbed native soil is 30 MPa/m.

All building floor slabs must be provided with a capillary moisture barrier and drainage layer. This is made by placing the concrete slab on a minimum 200 mm layer of 19 mm clear stone (OPSS.MUNI 1004) compacted by vibration to a dense state. The upper 50 mm of clear stone can be replaced with 19 mm crusher run limestone for a working surface. The clear stone and a cohesionless subgrade must be separated by a geotextile such as Terrafix 270R (or approved equivalent) to prevent the migration of fines into the clear stone layer which could result in loss of support for the slab. Alternatively, Granular 'A' Type I (OPSS.MUNI 1010) compacted to 100% SPmdd can be utilized without the filter cloth.

For new structures that will be slab-on-grade with no basement levels, perimeter and underslab drainage at the foundation level is not required, provided that the underside of concrete slab is at least 200 mm above the prevailing grade of the site and the surrounding surfaces slope away from the building at a gradient of at least 2% to promote surface water run-off and to reduce groundwater infiltration adjacent to foundations. To minimize infiltration of surface water, the upper 150 mm of backfill could consist of less permeable, compacted soil such as clayey silt at the site.

4.6 Site Servicing

It is expected that the proposed commercial development will be serviced with municipal water, sanitary and storm sewers. Inverts were assumed to extend as deep as 3 m below the existing grade for the purposes of this report.



4.6.1 Bedding

The type of material and depth of granular bedding below the pipe will, to some extent, depend on the method of construction used by the contractor. Pipe bedding for flexible pipes should follow the requirements in Ontario Provincial Standard Drawing 802.010 or applicable municipal standards. Pipe bedding for rigid pipes should follow the requirements in Ontario Provincial Standard Drawings 802.030 to 802.032 or applicable municipal standards.

A subgrade consisting of the native glacial till or engineered fill will provide adequate support for pipes with the bedding requirements as laid out in the above referenced OPS drawings. Where disturbance of the trench base has occurred from groundwater seepage, construction traffic, etc., or if in-situ fill is present at the invert level, the material should be sub-excavated and replaced with suitably compacted granular fill. If weak zones are encountered, additional bedding materials and differing construction practices may be required and should be determined during construction. Any zones of organic soil should be sub-excavated and replaced with approved earth fill or imported granular material compacted to 95% SPmdd. Details on temporary groundwater control are provided in Section 5.2.

Regardless of whether flexible or rigid pipes are implemented, granular bedding and cover material should consist of a well graded, free draining material, such as Granular "A" (OPSS.MUNI 1010). All granular bedding must be compacted to a minimum of 95% SPmdd.

4.6.2 Backfill

Excavated native inorganic cohesionless soil may be re-used as backfill in trenches, provided it is moisture conditioned so that the moisture content is within 2% of optimum. Additional soil compaction details are provided in Section 5.3. The backfill should be compacted to a minimum of 95% SPmdd. In confined areas the layer thickness will have to be reduced to utilize smaller compaction equipment efficiently or by using granular material instead of locally sourced fill. Any backfill that is frozen, contains a high percentage of organic material (topsoil, peat, etc.) or moisture, or has otherwise unsuitable deleterious inclusion should not be used as backfill. The maximum cobble or boulder size should not exceed half of the loose lift thickness (i.e., all particles with a diameter greater than 100 mm should be removed). Where cohesive soils are utilized as backfill a sheepsfoot compactor will be required. In general, excavated clayey silt soil from the site is not recommended for re-use as backfill because these soils have moisture contents above optimum. Reducing moisture in the cohesive soil is not practical and the compaction specifications cannot be achieved.

Where trenches are within the traveled portions of a roadway, backfill within the frost penetration depth of 1.2 m should consist of native, non-organic, excavated material consistent with the soils surrounding the trench. If this technique is not undertaken, then frequently problems arise with yearly differential frost heave movements between the trench backfill and the adjacent native soil. This would occur, for example, if imported granular material is used



to backfill trenches which is less susceptible to frost effects compared to the native soils on site. Alternatively, if different soil is used as the backfill due to issues with achieving compaction, a frost taper of 10H:1V can be implemented to help mitigate the potential for differential settlement and frost heave.

4.7 Pavement Design

The proposed development will have paved access, parking areas and Clark Boulevard, as shown on the concept plan Figure 2B.

4.7.1 Subgrade Preparation

The grading has not been completed at this time, however the building slab levels are noted to be at or just above the existing grade and as such, for purposes of this report, the road grade is also assumed to be at or just above the existing grade. Based on this the subgrade for the road is assumed to comprise engineered fill, the quality of which is unknown at this time, or the upper native cohesive soil which is highly frost susceptible.

The subgrade must be inspected and approved by the geotechnical engineer at the time of construction. If the subgrade does not comprise engineered fill, the exposed pavement subgrade should be compacted to a minimum 95% SPmdd. If any soft or weak subgrade areas are identified, or if there are areas containing excessive amounts of moisture or deleterious/organic material, they must be locally sub-excavated and backfilled with approved clean earth fill or imported granular material and compacted to a minimum of 95% SPmdd.

The long-term performance of the pavement structure is highly dependent upon the subgrade support conditions. Stringent construction control procedures must be maintained to ensure that uniform subgrade moisture and density conditions are achieved as much as possible when fill is placed, and the natural subgrade is not disturbed or weakened after it is exposed.

4.7.2 Drainage

Control of surface water is an important factor in achieving a good pavement life. The need for adequate subgrade drainage cannot be over-emphasized. The subgrade must be free of depressions and sloped (at a minimum grade of 2 percent) to provide effective drainage toward subgrade drains. Grading adjacent to pavement areas should be designed to ensure that water is not allowed to pond adjacent to the outside edges of the pavement.

Continuous pavement subdrains should be provided along both sides of the roadways and around the perimeter of parking areas and drained into respective catchbasins to facilitate drainage of the subgrade and the granular materials. The subdrain invert should be maintained at least 0.3 m below subgrade level. To minimize the problems of differential movement between the pavement and catchbasins/manholes due to frost action, the backfill around the



structures should consist of free-draining OPSS Granular B. Typical pavement drainage details are provided in Appendix C.

4.7.3 Pavement Structure

The two different types of pavements for this project are defined below:

- <u>Light duty</u>: Includes parking lots which will not see frequent heavy traffic loads such as buses, delivery or fire trucks, etc., and will mostly service small vehicles such as cars or pickup trucks. In this case this design should be used for the parking lot used by the cars
- <u>Heavy Duty</u>: Includes access and driveways (Clark Boulevard) which are designated fire truck routes, or will see frequent heavy traffic loads such as school buses, delivery or garbage trucks, etc. In this case this design should be used for the driveway used by trucks and loading aprons.

The industry pavement design methods are based on a design life of 15 to 20 years for typical weather conditions depending on actual traffic volumes. The following pavement thickness designs are provided on the above noted considerations and conservatively based on the highly frost susceptible native soil. When the subgrade is confirmed, the designs can be finalized.

Pavement Layer	Compaction	Minimum C Thick	
	Requirements	Light-Duty	Heavy-Duty
<u>Surface Course Asphaltic Concrete:</u> HL3 (OPSS 1150) with PG 58-28 Asphalt Cement (OPSS.MUNI 1101)	OPSS 310	40 mm	40 mm
<u>Binder Course Asphaltic Concrete:</u> HL8 (OPSS 1150) with PG 58-28 Asphalt Cement (OPSS.MUNI 1101)	OPSS 310	50 mm	80 mm
<u>Base Course:</u> Granular A (OPSS.MUNI 1010)	100% Standard Proctor Maximum Dry Density (ASTM-D698)	150 mm	150 mm
<u>Subbase Course:</u> Granular B Type I or II (OPSS.MUNI 1010)	100% Standard Proctor Maximum Dry Density (ASTM- D698)	450 mm	600 mm

The granular materials should be placed in lifts 200 mm thick or less and be compacted to a minimum of 100% SPmdd for both granular base and subbase. Asphalt materials should be rolled and compacted as per OPSS 310. The granular and asphalt pavement materials and their placement should conform to OPSS 310, 501, 1010 and 1150.



If the pavement construction occurs in wet, winter or inclement weather, it may be necessary to provide additional subgrade support for heavy construction traffic by increasing the thickness of the granular subbase, base or both. Further, traffic areas for construction equipment may experience unstable subgrade conditions. These areas may be stabilized utilizing additional thickness of granular materials or geogrid materials.

It should be noted that in addition to adherence of the above pavement design recommendations, a close control on the pavement construction process will also be required in order to obtain the desired pavement life. Therefore, it is recommended that regular inspection and testing should be conducted during the pavement construction to confirm material quality, thickness, and to ensure adequate compaction.

Frost tapers of 10H:1V should be implemented between areas of differing pavement thickness and tie-in areas to existing pavement.

Smooth transitions are required in all areas where the new pavement meets the existing asphalt surface (existing road/Clark Boulevard). Asphalt joints shall follow OPSS.MUNI 310. Longitudinal asphalt joints should be milled into the existing asphalt a minimum of 0.5 m for each lift. Transvers joint shall be milled into the existing asphalt a minimum 0.5 m for each lift. Successive joints should be staggered.

4.8 Stormwater Management Ponds

SWM ponds are currently proposed at the south side of all three buildings. An underground SWM pond is proposed at Building 1 and two at grade SWM ponds are proposed at Buildings 2 and 3. Details of the ponds were not known at the time of this report.

Boreholes 37, 39 and 41 were drilled in the area of the proposed underground SWM pond south of Building 1 and revealed topsoil and fill to about 0.8 m depth, over clayey sandy silt to about 2.3 to 4.6 m depth. The glacial till was underlying the clayey sandy silt to the depth of the boreholes. The stabilized groundwater was measured at 1.2 m depth.

Boreholes 22, 23, 42, 44, 45 and 47 were advanced in the area of the SWM pond south of Building 2. The boreholes showed topsoil and fill to typically 0.8 m depth, overlying clayey silt and clay and silt to 1.5 to 3.0 m depth. A local sand layer was below the clayey silt in one borehole. Underlying the clayey silt/clay and silt and local sand, the glacial till deposit was present to 5.0 to 8.1 m depth of the boreholes. The stabilized groundwater was measured at 0.9 to 2.7 m depth.

Boreholes 51 and 52 were drilled in the area of the SWM pond south of Building 3. The topsoil and fill to 0.8 m depth were over the clay and silt/sandy clayey silt, that extended to 2.3 to 3.0 m depth. The till was below the cohesive units. Stabilized groundwater was assumed to be near about 1 m below grade (well was destroyed below water levels could be taken).



4.8.1 General Construction Considerations

Excavation and temporary ground water control construction considerations are provided with Section 5.1 of this report and apply to the construction of the SWM pond.

The steepest recommended pond slope inclination is 5H:1V and should follow the design guidelines of the local municipality.

It is recommended that any piping or trenching in the area of the pond should be provided with seepage cut-off collars (clay plugs, concrete plugs, or other barriers) to protect against water seepage through the pipe bedding and backfill.

Pond berms above grade will have to be constructed as engineered fill, constructed as described earlier in the report.

4.8.2 Pond Slope Surface Treatment

The final slope surface and all bare or exposed areas (where applicable) should be provided with suitable vegetation cover or erosion protection. The sloped surface should be provided with a layer of topsoil (minimum 100 mm thick) and should be hydro-seeded with a grass mixture and mulch. If seeded, during the first 2 to 3 years, the surface cover of topsoil and seeding may require periodic maintenance until the vegetation becomes well established. It is recommended that erosion netting/erosion control blankets be staked on the slope surface for erosion protection (including the inside slope above the water level).

4.8.3 Liner Considerations

Depending on the type of SWM pond that is planned, a liner may be required if a permanent pool is proposed. The liner should be placed along the entire pond bottom and extend a minimum of 1.0 m above the permanent pool elevation. The liner may consist of a natural soil material (such as clay), a synthetic membrane liner (such as a High-Density Polyethylene, Geosynthetic Clay Liner, or PVC), a concrete liner, or a combination thereof. Details can be provided when the design has progressed.

The liner system must be designed to withstand uplift pressure due to hydrostatic head at the base of the liner for the worse-case condition when the pond is emptied for cleaning and maintenance activities. Uplift pressure can be assessed and reviewed when design details are established. A gravel/rip rap protection layer should also be considered for the liner when the pond is cleaned out in the future.



4.8.4 Infiltration Considerations

It is understood the pond south of Building 1 will be an underground infiltration area. The base of the facility has not been determined however considering the floor slab elevation of the building, the adjacent parking area and burial of an approximate 1 m high facility, the base is assumed to be about 2.5 m below the proposed grade near Elev. 189.0 +/-. Based on the boreholes the base of the facility would be in the clayey silt unit, possibly within the upper portion of the till.

Typical design of infiltration facilities has the base of the feature a minimum of 1 above the groundwater table. As such, the base of the infiltration gallery may need to be raised based on the groundwater level measurements, or may not be practical/possible. The infiltration rate provided below is not applicable below the groundwater table.

The hydraulic conductivity of the soils encountered on site is generally estimated to be less than $1 \ge 10^{-7}$ m/s based on the grain size curves and does not consider the density of the soil. The resulting unfactored infiltration rate is assumed to be less than 10 mm/hr.

Appendix C of "Low Impact Development Stormwater Management and Planning Design Guide" (Version 1.0, 2010, by CVC and TRCA) suggests safety factors to be applied to infiltration rates. The safety factor applicable to the site is expected to be 2.5 but this must be confirmed during detailed design. Once the final location and elevation of LID measures are known, in-situ infiltration testing using the Guelph Permeameter could be completed to refine the infiltration rates.



5. Constructability Considerations

5.1 Excavations

At this time, excavations for the project site are anticipated to be about 1.0 to 3.0 m below existing grade to account for SWM ponds, buildings and servicing. Below the surficial topsoil and fill, excavations are anticipated to encounter cohesive soil units (clayey silt/sandy clayey silt/clay and silt), discontinuous cohesionless soil layers (sandy silt/silt/sand and silt, sand or sand and gravel) over a major glacial till deposit. Harder digging can be expected in the glacial till. Cobbles and boulders can be expected in the glacial till.

Excavations must be carried out in accordance with the Occupational Health and Safety Act, Ontario Regulation 213/91 (as amended), Construction Projects, Part III - Excavations, Section 222 through 242. Where workers must enter a trench or excavation the soil must be suitably sloped and/or braced in accordance with the OHSA. These regulations designate four (4) broad classifications of soils to stipulate appropriate measures for excavation safety. If more than one soil type is encountered in an excavation, the most conservative soil type must be followed for sloping the sidewalls of the excavation. Excavations for the site should be completed considering a Type 3 soil geometry, 1H:1V from the base of the excavation, assuming that the soils are dewatered prior to excavation.

Excavation sidewalls will need to be continuously reviewed for evidence of instability and ground water seepage, particularly following periods of heavy rain or thawing. When required, remedial action must be taken to ensure the continued stability of excavation slopes and the safety of the workers.

Minimum support system requirements for steeper excavations are stipulated in Sections 235 through 238 and 241 of the OHSA and include provisions for timbering, shoring and moveable trench boxes. To reduce the potential for instability of the trench excavations, materials excavated from the service trenches and/or other fill materials or heavy equipment should not be placed near the crest of the trench excavations.

It is important to note that soil encountered in the construction excavations may vary significantly across the site. Our preliminary soil classifications are based solely on the materials encountered in the boreholes advanced on site. The contractor should verify that similar conditions exist throughout the proposed area of excavation. If different subsurface conditions are encountered at the time of construction, we recommend that GEI be contacted immediately to evaluate the conditions encountered.



5.2 Temporary Construction Groundwater Control

As noted above, excavation is envisioned to extend to about 1.0 to 3.0 m depth for the project.

The stabilized groundwater levels in the monitoring wells are typically within about 1 m of the ground surface corresponding to Elev. 185.8 to 191.5. This is believed to reflect perched water in the fill above the clayey silt/sandy clayey silt/clay and silt and till.

Excavation into/below the local perched water, wet seams or local wet sand layers will not yield large seepage volumes due to the nature of the predominate soil at the site.

The exact scenario where certain groundwater control techniques will work are directly correlated to how coarse/fine the native soils are in an excavation, and both the lateral and vertical extent of the wet cohesionless deposits encountered as noted above. If the groundwater table is not controlled during construction, the base of the excavations will be unstable, leading to difficulties in excavating and placement of pipes, footings or engineered fill, and providing safety for the workers.

Conventional sump pumping should suffice to control ground water seepage for excavations that extend to 3.0 m depth below existing grade. Locally in sandier zones several sumps or keg wells may be required.

It is recommended to carry out the work during the dry time of the year when the ground water table is lowest, to mitigate groundwater control measures. Also reducing the size of the excavation that is open at any one time will aid in reducing groundwater control requirements.

Based on the above, a Permit-to-Take-Water (PTTW) is not required. Registry on the Environmental Activity and Sector Registry (EASR) system may be a prudent action to allow for areas of greater groundwater seepage with no work stoppage.

GEI's hydrogeological study under a separate cover provides further details regarding water taking analysis, regulatory requirements, impact assessments, monitoring plans, etc. for the site and must be referenced for groundwater control considerations.

5.3 Compaction Specifications

Standard Proctor Maximum Dry Density the specification to indicate the degree to which soil or aggregate is compacted. To achieve the specified SPmdd as indicated in this report, all soils or aggregates must be placed in lift thicknesses no greater than 200 mm. If this is not the case, only the upper portion of the lift will be adequately compacted, and the lower portion of the lift has a high probability of not meeting compaction specifications. In addition, industry standard equipment used to determine the degree of compaction consists of nuclear densometers. These devices have an inherent limitation in that they cannot test beyond



300 mm in depth, and so the degree of compaction beyond this depth cannot be quantitatively determined.

Along with lift thickness, ensuring that the soil or aggregate is within 2% of its optimum moisture content ensures that the specified compaction can be reached. If the soil or aggregate is too dry/wet, it is either very difficult or impossible to reach the specified compaction. This is especially true for when higher compaction specifications such as 98% and 100% SPmdd are required.

Based on our review of the soil types encountered in the boreholes with associated moisture contents, the soils at this site above the ground water table are near optimum and the soil below the ground water table is wet of optimum.

Moisture can be increased by adding water and mixing the soil prior to re-use, blending the soil with wetter material, or by importing soil to the site that is at optimum and can be readily compacted.

Moisture can be reduced by tilling or spreading out the soil to dry or blending it with drier material. In-situ moisture contents can change based on the season and local groundwater levels and can also change for stockpiled material due to precipitation. Zones of the fine-grained soil beneath the site have very high moisture contents and moisture conditioning may be difficult to accomplish.

Excavated clayey soil from the site should not be re-used as engineered fill or common backfill because these soils have moisture contents well above optimum. Reducing moisture in the cohesive soil is likely not practical and the compaction specifications will not be achieved.

In addition to the above compaction specifications, in any areas where compacted fill will be placed over the exposed native soil subgrade, any loose, soft, wet, organic or unstable areas should be sub-excavated, and backfilled with clean earth fill or Granular 'B' (OPSS.MUNI 1010) compacted to a minimum of 95% SPmdd. This recommendation applies to site servicing and pavement subgrades. Where structures/buildings require upfilling beneath the structure the fill should be compacted to 100% SPmdd.

5.4 Quality Verification Services

On-site quality verification services are an integral part of the geotechnical design function, and for foundations, engineered fill and retaining walls, are required under the Ontario Building Code. Quality verification services are used to confirm that construction is being conducted in general conformance with the requirements as outlined in the drawings, reports and specifications prepared for the proposed development.

GEI Consultants can provide all the on-site quality verification services outlined below:



- The subgrade for shallow foundations for commercial buildings must be field reviewed by the geotechnical engineer as required by the municipal regulating authority.
- Installation of retaining structures over 1.0 m high and related backfilling operations must be field reviewed on a continuous basis by the geotechnical engineer as required in the OBC.
- Full-time monitoring, testing and inspection of engineered fill placement is required by the geotechnical engineer per the OBC.
- Part-time monitoring of the subgrade support capabilities, material quality, lift thickness, moisture content, degree of compaction, etc. is recommended for the following areas to ensure the recommendations within this report are followed and they perform adequately in the long-term;
 - Slab-on-grades;
 - Pavement structure (granular and asphalt); and
 - Bedding/backfilling of site servicing.
- Testing of the concrete (compressive strength, slump, air content, etc.) and testing of the asphalt (asphalt content and gradation) are recommended to ensure that the quality of the materials being brought to site meet the requirements of the project.

5.5 Site Work

The soils found at this site may become weakened when subjected to traffic, particularly when wet. If there is site work carried out during periods of wet weather, then it can be expected that the subgrade will be disturbed unless an adequate granular working surface is provided to protect the integrity of the subgrade soils from construction traffic. Subgrade preparation works cannot be adequately accomplished during wet weather and the project must be scheduled accordingly. The disturbance caused by the traffic can result in the removal of disturbed soil and use of granular fill material for site restoration or underfloor fill that is not intrinsic to the project requirements.

The most severe loading conditions on the subgrade may occur during construction. Consequently, special provisions such as end dumping and forward spreading of earth and aggregate fills, restricted construction lanes, and half-loads during paving and other work may be required, especially if construction is carried out during unfavourable weather.

If construction proceeds during freezing weather conditions, adequate temporary frost protection for the founding subgrade and concrete must be provided. The soil at this site is susceptible to frost damage. Consideration must be given to frost effects, such as heave or softening, on exposed soil surfaces in the context of this particular project development.



6. Limitations and Conclusions

6.1 Limitations

The recommendations and comments provided are necessarily on-going as new information of underground conditions becomes available. More specific information with respect to the conditions between samples, or the lateral and vertical extent of materials may become apparent during excavation operations. The interpretation of the borehole information must, therefore, be validated during excavation operations. Consequently, conditions not observed during this investigation may become apparent. Should this occur, GEI should be contacted to assess the situation and additional testing and reporting may be required.

GEI should be retained for a general review of the final design drawings and specifications to verify that this report has been properly interpreted and implemented. If not accorded the privilege of making this review, GEI will assume no responsibility for interpretation of the recommendations in the report.

The comments given in this report are intended only for the guidance of the design engineers. The number of boreholes required to determine the localized underground conditions between boreholes affecting construction costs, techniques, sequencing, equipment, scheduling, etc. could be greater than has been carried out for design purposes. Contractors bidding on or undertaking the works should, in this light, decide on their own investigations, as well as their own interpretations of the factual borehole results, so that they may draw their own conclusions as to how the subsurface conditions may affect them.

This report was authorized by, and prepared by GEI for, the account of Anatolia Investments Corporation (as provided in the signed Standard Professional Services Agreement). Any use which a third party makes of this report, or any reliance on or decisions to be made based on it, are the responsibility of such third parties. GEI accepts no responsibility for damages, if any, suffered by any third party as a result of decisions made or actions based on this project.



6.2 Conclusion

It is recognized that municipal/regional governing bodies, in their capacity as the planning and building authority under Provincial statues, will make use of and rely upon this report, cognizant of the limitations thereof, both as are expressed and implied.

We trust this report is complete within our terms of reference, and the information presented is sufficient for your present purposes. If you have any questions, or when we may be of further assistance, please do not hesitate to contact our office.

Yours Truly,

GEI Consultants

Prepared By:

Reviewed By:



Nohd Ragen

Mohammed Razeen Geotechnical E.I.T.

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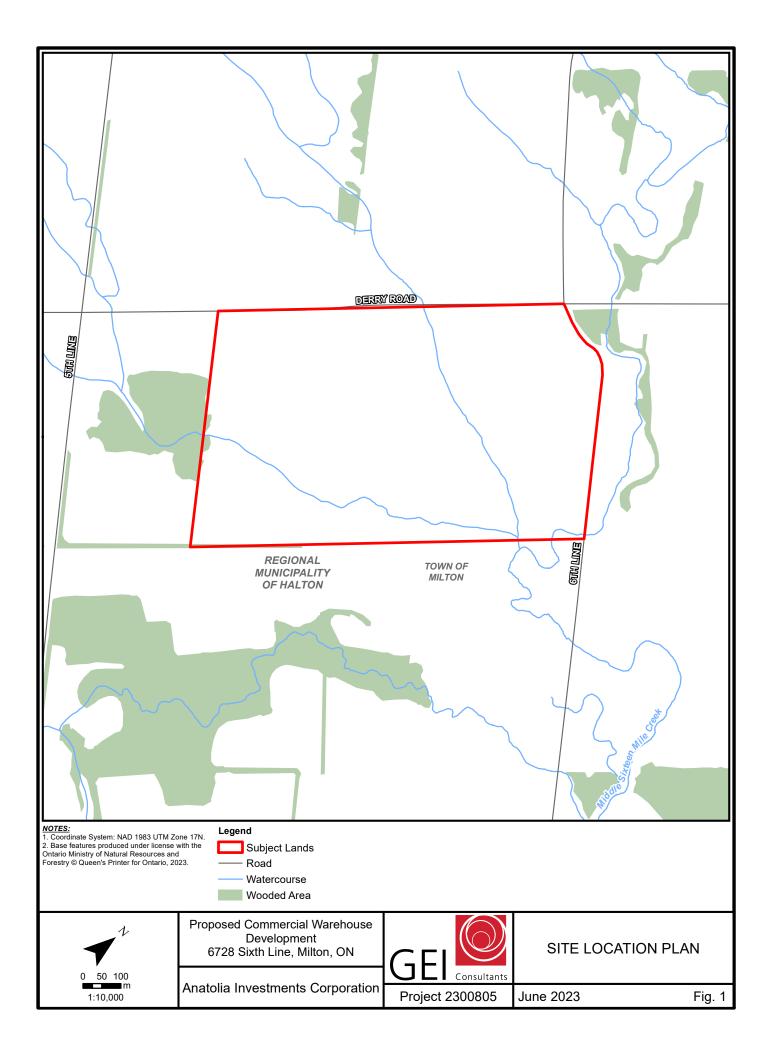
Geoffrey R. White, P.Eng. Geotechnical Practice Lead

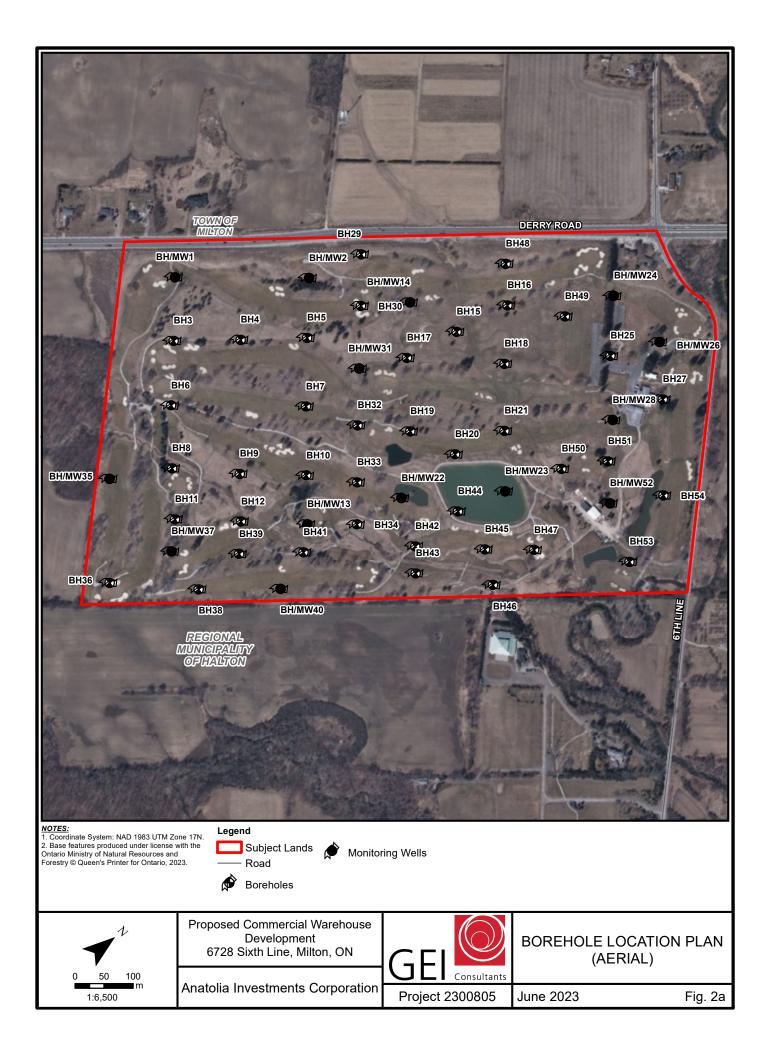


Figures

Site Location Plan Borehole Location Plans









Geotechnical Investigation Proposed Commercial Warehouse Development 6728 Sixth Line, Milton, Ontario Project No. 2300805, June 14, 2023



Borehole Logs



REC

RECORD	OF BOREHO	LE	No	. 1										
Project Number:	2300805												G	
Project Client:	Anatolia												U	Consultants
Project Name:	Proposed Commerce	ial W	areho	ouse			Drillin	ng Method:	Solid Stem Au	igers		Drilling Machine:	Track M	lount
Project Location:	6728 Sixth Line, Mil	ton, C	ON				Logg	ed By:	SDP	_ Nor	thing:	4821286	Date St	tarted: Apr 24/23
Drilling Location:	See Borehole Locat	ion P	lan				Revie	ewed By:	GW	_ Eas	ting:	594889.1	Date C	ompleted: Apr 24/23
Local Benchmark														
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	T : (705) 719-7994 ww.geiconsultants.com a qualified geotect	(5) 719-7994 Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from Scale: 1:75																					

RECORD OF BOREHOLE No. 2



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Di
Project Location:	6728 Sixth Line, Milton, ON	Lo
Drilling Location:	See Borehole Location Plan	Re
Local Benchmark:		

	Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
	Logged By:	SDP	Northing:	4821460	Date Started:	Apr 24/23
_	Reviewed By:	GW	Easting:	595031.6	Date Completed:	Apr 24/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING						STING			LAE	TES	TING			6	OMN		
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	T : (705) 719-7994 Www.geiconsultants.com a qualified geoted	hole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from alified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was											Scale Page:									

RECORD OF BOREHOLE No. 3



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
•	

Drilling Method:	Solid Stem Aug	jers	Drilling Machine:	Track Mount					
Logged By:	SDP	Northing:	4821210	Date Started:	Apr 24/23				
Reviewed By:	GW	Easting:	5945031.6	Date Completed:	Apr 24/23				

Local Benchmark:

LITHOLOGY PROFILE		SOIL SAMPLING				FIELD TESTING		LA		COMMENTS									
ogy Plot	DESCRIPTION 192.1 10.2 FILL: Clayey silt, some sand, trace organics, firm, brown, moist		Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	Shear Strength Testing (kPa) X Other Test + Pocket Penetrometer ▲ Field Vane (Intact) △ Field Vane (Remolded) 40 80 120 Penetration Testing		Combustible Organic Vapour (ppm) Combustible Organic Vapour (%LEL) Total Organic Vapour (ppm) 100 200 300 400 Atterberg Limits PL LL				Instrumentation Installation	& GRAIN SIZE DISTRIBUTION (%)				
Lithol			Samp	Samp				O SPT ● DCP1 10 20 30			ter Content 20		LL 10	Instru Install	GR	SA	SI	CL	
	0.2 TOPSOIL: 125 mm ^{191.9} FILL: Clayey silt, some sand, trace organics, firm, brown, moist _{191.3}	SS	1	85	5	0	- 192	0 5			0 19								
	CLAYEY SILT: Some sand, trace gravel, stiff, brown, moist	SS	2	100	9	1.5 - 14 25 3 -	- - - - - - - - - - - - - - - - - - -	9 0			24 O								
		SS	3	100	14			14 0			5								
i H	2.3 189.8 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact to dense, brown, wet to moist	66	4	100	25			250		70					First Water Strike SS4				
		SS	5	100	32			320		10									
					02	-	_												
						4.5-	- 187.5												
000 - 000 1 - 00 - 00 - 00 - 00 - 00 - 0	Some gravel, brownish grey	SS	6	100	24		101.0	24 0		9 C									
						-	-	 											
			7	100	19	6-	- 186	19 🗘		10									
000000 000000 000000							-												
0 20 20 20						7.5	184.5			10									
	8.1 183.9	SS	8	100	16			16 0		10									
	Borehole Terminated at 8.1 m																		
	l																		
647 Welham Road, Unit 14 Barrie, Ontario L4N 0B7			indwater depth encountered on completion of drilling: 7.6 m. 🔶 Cave depth after auger removal: Open																
			undwater depth observed on: Groundwater Elevation:								Socia	4 .75							
WWW.geiconsultants.com a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was									Scale Page										



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method:	Solid Stem Aug	jers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821304	Date Started:	Apr 24/23
Reviewed By:	GW	Easting:	594968.5	Date Completed:	Apr 24/23

	LITHOLOGY PROFILE	SO		MPL	ING				D TESTIN			LAB	TES	TING			c	OMN	IENT	
ology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 X Other Te + Pocket P ▲ Field Var △ Field Var 40 	rength Testing (st enetrometer ne (Intact) ne (Remolded) 80 120 etration Testing	160		Total Org	ble Orgai	nic Vapo our (ppm 00 4	ur (%LEL	Instrumentation Installation	6	8 RAIN STRIE	R N SIZI BUTIC %)	E
Litho	0.0 191.5		Sam	Reco	SPT			O SPT	DCPT 20 30	40			Content		40	Instr Insta	GR	SA	SI	CL
	0.2 TOPSOIL: 150 mm ^{191.4} FILL: Clayey silt, some sand, trace gravel, firm, grey, moist _{190.8}	ss	1	60	6	0	-	0 6					0 22							
	CLAYEY SILT: Some sand, trace gravel, very stiff, brown, wet	SS	2	100	18	-	- 190.5	18 9				16 0					First \	Water :	Strike	SS2
		SS	3	100	20	1.5 –	-	20				14 O								
	2.3 189.3 SANDY SILT GLACIAL TILL: Trace clay, trace gravel, inferred cobbles and	60	4	65	48	-	- 189		``	48 (12 O								
	boulders, dense to compact, brown, moist to wet					3-	-					10								
		SS	5	100	47		-			47 ¢		Ő								
the second s						-	- 187.5		/											
		SS	6	100	25	4.5 -	-		250			2								
91 B.	5.0 186.5 Borehole Terminated at 5.0 m																			
		ndwat	ter de	pth er	ncount	ered o	n compl	etion of drilli	ng: Dry		Cave d	lepth a	fter au	ger rei	moval:	Open	•			
	arrie, Ontario L4N 0B7				oserve						Ground									
w	T: (705) 719-7994 ww.geiconsultants.com ww.geiconsultants.com	hnical	enginee	r. Also,	, boreho	le inform	ation sho	uld be read in co							ice from				:1:75 :1 of 1	



Project Number:	2300805	
Project Client:	Anatolia	_
Project Name:	Proposed Commercial Warehouse	Drilling I
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Reviewe
Local Benchmark:		

Drilling Method:	Hollow Stem	Augers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821393	Date Started:	Apr 21/23
Reviewed By:	GW	Easting:	595046.6	Date Completed:	Apr 21/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING		FIELD TESTING	LAB TESTING		COMMENTS		
ter and the second s	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m) ELEVATION (m)	Shear Strength Testing (kPa) X Other Test + Pocket Penetrometer ▲ Field Vane (Intact) △ Field Vane (Remolded) 40 80 120 160 Penetration Testing ○ SPT ● DCPT		Instrumentation Installation	& GRAIN SIZE DISTRIBUTION (%) GR SA SI CL		
	8.9	ം ss	ഗ് 1	₩ 75	່ນັ້ 10	0 - 190.8	10 20 30 40	0 10 20 30 40 0 18	ËË			
	0.8 organics, stiff, brown, wet 189.8 CLAYEY SILT: Some sand, trace gravel, stiff, brown, moist to wet	SS	2	80	13	-	13 \	18	F	irst Water Strike SS2		
		SS	3	85	13	1.5 - 189	134	27				
	2.3 188.3 SANDY SILT GLACIAL TILL: Some clay, trace to some gravel, inferred	SS	4	90	33	-	330	10				
	cobbles and boulders, dense, brown to brownish grey, moist	ss	5	85	43	3 - 187.5		9				
					10	Ē						
						4.5 - 186		7				
Ц. Ц	5.0 185.6 Borehole Terminated at 5.0 m	SS	6	100	38		38 0	0				
		ndwat	er de	oth er	icount	ered on com	ppletion of drilling: 3.0 m.	Cave depth after auger removal: 3	3.9 m.			
	7 Welham Road, Unit 14 arrie, Ontario L4N 0B7 T : (705) 719-7994 Borehole details (-				standing of all potential conditions presen	Groundwater Elevation:		0		
w	ww.geiconsultants.com a qualified geoted	Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'. Page: 1 of 1										



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method:	Solid Stem Aug	gers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821393	Date Started:	Apr 24/23
Reviewed By:	GW	Easting:	595061.7	Date Completed:	Apr 24/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING			Τ	FIELD TES			LAB	B TES	TING			c	OMN		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)		Shear Strength Test → Other Test + Pocket Penetromet ▲ Field Vane (Intact) △ Field Vane (Remol 40 80 12 Penetration Te	er ded) 20 160	_ ▲ (Combusti Fotal Org 00 2	ible Organ ible Organ anic Vapo 200 3 Prberg Lim	nic Vapou our (ppm 00 4	ur (%LEL)	Instrumentation Installation	0	8 GRAIN STRIE	k I SIZI	E
Litho	0.0 191.3		Sam	Rec	SPT	<u> </u>	E		O SPT ● DCI 10 20 3	рт 0 40	Ċ) Water	r Content		10	Instr Insta	GR	SA	SI	CL
	0.2 TOPSOIL: 150 mm ^{191.2} FILL: Clayey silt, some sand, trace organics, firm, brown, moist _{190.6}	ss	1	85	5	0	-		5				0 22							
	CLAYEY SILT: Some sand, trace gravel, stiff to very stiff, brown, moist	SS	2	100	12		- 190.5		120				20							
		SS	3	100	19	1.5—	_		190			17 0								
	2.3 189.0 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and		4	100	41		- 189			410		13 O					First \	Water :	Strike	SS4
	boulders, dense to compcat, brown to brownish grey, wet to moist					3-	-	╞)								
		SS	5	100	40					40 ¢ /										
						-	- 187.5													
						4.5 —	-		/		1	0								
		SS	6	100	23				23 Ý 			Ĭ								
						-	- 186													
						6-					_									
		ss	7	100	22				22 🔶			11 P								
0.00 0.01 0.01 0						-	- 184.5													
									Ì											
1000000	Some gravel	SS	8	100	28	7.5	7		280		9	2								
12162	8.1 183.2 Borehole Terminated at 8.1 m	·																		
	GEI CONSULTANTS	ndwat	er de	pth en	count	tered or	n compl	oleti	ion of drilling: 7.6	m	Cave d	epth a	fter au	ger rer	noval:	Open	-			
647	Y Welham Road, Unit 14	ndwat	er de	pth ob	serve	ed on:					Ground	lwater	Elevat	ion:						
w	T : (705) 719-7994 ww.geiconsultants.com a qualified geotec commissioned an	hnical e	enginee	r. Also,	boreho	ole inform	ation sho	ould	ling of all potential con I be read in conjunction	ditions preser with the geo	nt and requi	ire interp eport for	vetative which it	assistan was	ce from			Scale Page:		



Project Number:	2300805		
Project Client:	Anatolia		
Project Name:	Proposed Commer	cial Warehouse	Dr
Project Location:	6728 Sixth Line, Mil	ton, ON	Lo
Drilling Location:	See Borehole Loca	tion Plan	Re
Local Benchmark:			
LITHOLOG	GY PROFILE	SOIL SAMPLING	

Drilling Method:	Hollow Stem A	ugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821327	Date Started:	Apr 25/23
Reviewed By:	GW	Easting:	595205.4	Date Completed:	Apr 25/23

	LITHOLOGY PROFILE	80	IL SA	MDI	ING	ĺ		FIELD TESTIN			AB TEST			
		30	12 34					Shear Strength Testing (AD IESI	ING		COMMENTS
ter and the second s	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	X Other Test + Pocket Penetrometer Field Vane (Intact) A Field Vane (Remolded) 40 120 Penetration Testing O SPT ● DCPT	160	▲ Com ◇ Tota 100 PL		its LL	Instrumentation Installation	& GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
	0.0 190 0.2 TOPSOIL: 150 mm ¹⁹⁰ FILL: Clayey silt, some sand, trace	5	<u>ທັ</u> 1	™ 75	5 10				40	10	20 3 (3	0 40	드드	
	organics, stiff, brown, moist CLAYEY SILT: Some sand, stiff, greyish brown, moist	5 SS	2	100	13			130			19 0	0		
	greyian blown, moist	SS	3	100	13	1.5 —	189	13 4			27 0			
	2.3 188. SANDY SILT GLACIAL TILL: Some		3	100	13	-				6				
	clay, trace gravel, inferred cobbles and boulders, dense, dark brown, moist	d ss	4	100	33	3-	187.5	330	、 	6 0				
100 000 200 000 200 000		SS	5	100	43	[°] ¥			43 \overline{43}	9				
00000							186		/					
90	Wet 5.0 185.	3 SS	6	65	38	4.5 —		38	6	11 0			F	First Water Strike SS6 9 29 44 18
PP4 4-50	Borehole Terminated at 5.0 m													
		undwa	ter de	pth er	ncoun	tered on c	compl	letion of drilling: 3.2 m.	<u> </u>	Cave dept	th after aug	ger removal:	Open	
	arrie, Ontario L4N 0B7	undwa					ndoret-	anding of all notestial as a differen	0.00000		ater Elevati			-
w	T : (705) 719-7994 ww.geiconsultants.com commissioned a	chnical	enginee	er. Also	, boreho	ole information	on sho	anding of all potential condition ould be read in conjunction with j'.	s presen the geo	technical repo	rt for which it	was		Scale: 1 :75 Page: 1 of 1



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	D
Project Location:	6728 Sixth Line, Milton, ON	Lo
Drilling Location:	See Borehole Location Plan	R

	Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount						
	Logged By:	SDP	Northing:	4821074	Date Started:	Apr 27/23					
_	Reviewed By:	GW	Easting:	595147.5	Date Completed:	Apr 27/23					

Local Benchmark: _____

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				FIELD TES	-		LAB	TES	TING		со		OMN	IFNT	s						
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	(W) T	ELEVATION (m)	> + 2	Field Vane (Intact) Field Vane (Remole 40 80 12	er led) 1 <u>60 160</u>		Combustible Organic Vapour (%LEL Total Organic Vapour (ppm) 100 200 300 400 Atterberg Limits				A Ormaharatible Ormania Manager (0/1 E1)			Instrumentation Installation	& GRAIN SIZE						
Litholo	8.9 180.5	Sample	Sample	Recov	SPT "N	DEPTH (m)	ELEV	0	Penetration Tes) SPT ● DCF 10 20 30) Water	Content	(%)	⊣ .∟ ю	Instrun Installa	GR	SA	SI	CL						
	FILL: Clayey silt, some sand, firm,	ss	1	75	7	0	190.5		9	<u> </u>		2	<u>)</u> 0					I	I							
	0.8 Brown, model 189.7 CLAYEY SILT: Some sand, stiff, brown, moist		2	100	14	-	-		14 Q			15 O														
	1.5 189.0					1.5 —	- 189				_		22				Circt \	Notor	Strike	662						
	SANDY SILT GLACIAL TILL: Some clay, inferred cobbles and boulders, compact to dense, brown, wet to moist	SS	3	100	28	_	_		280				23 O				FIRSU	Water	Sirike	553						
		SS	4	100	38					380		0														
		SS	5	100	28	3—	- 187.5		28 ợ	/																
100000 100000 100000						-	-																			
						4.5 —	- 186	-	j j		1	0														
		SS	6	100	23	-	_		23 ¢																	
									/																	
	6.1 184.4 CLAYEY SANDY SILT GLACIAL TILL:	SS	7	100	16	6-	- 184.5		/			15					1	22	55	22						
	Trace gravel, inferred cobbles and boulders, compact to dense, brown to brownish grey, moist				-	-	-																			
						<u> </u>	Z = 183																			
		SS	8	100	43	7.5-	- 183			430	9															
renes	8.1 182.4 Borehole Terminated at 8.1 m																									
		ndwat	er de	pth en	icount	tered or	n comp	oletic	on of drilling: 7.4	m	Cave d	epth af	ter auç	ger rer	noval:	Open										
	7 Welham Road, Unit 14 arrie, Ontario L4N 0B7							han -!!			Ground															
w	T : (705) 719-7994 ww.geiconsultants.com a qualified geotec commissioned an	hnical e	enginee	r. Also,	boreho	ole inform	ation sho	ould b	ng of all potential conc pe read in conjunction	with the geot	and requi	eport for	which it	assistan was	ce from			Scale	:1:75							



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commer	cial Warehouse
Project Location:	6728 Sixth Line, Mi	lton, ON
Drilling Location:	See Borehole Loca	tion Plan
Local Benchmark:		
LITHOLOG	BY PROFILE	SOIL SAMPLING

Drilling Method:	Hollow Stem Au	ugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821164	Date Started:	Apr 27/23
Reviewed By:	GW	Easting:	595225.7	Date Completed:	Apr 27/23

	LITHOLOGY PROFILE	SO		MPL	ING						TING			LAB	TES	TING			COMMENT		s	
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	$\overset{\times}{\overset{+}{\vartriangle}}$	SPT	t netrome e (Intact) e (Remol 0 12 ration Te • DC	ter ded) 20 16 sting	0		Combusti Total Org 00 2 Atte	ble Organ anic Vap 00 3 rberg Lin	nic Vapo our (ppm 00 4 nits (%)	ur (ppm) ur (%LEL)) 	Instrumentation Installation	0	a GRAII STRI	& N SIZ BUTI(%)	E
Ĭ	00 TOPSOIL: 100 mm CLAYEY SILT: Some sand, trace gravel, firm to very stiff, brown, moist to	ss	1	90	7	0		C 7		03	0 40	<u>,</u>			0 0 21					<u>I</u>	1	
	wet 1.1 189.3 SANDY SILT GLACIAL TILL: Some	ss	2	85	17				179					14 O								
	clay, trace gravel, inferred cobbles and boulders, compact to very dense, brown, moist	SS	3	100	23	1.5 —	- 189		23	ν γ				18 0	8				First \	Water	Strike	SS3
		SS	4	100	40		-				40 ¢	>		0								
		SS	5	100	51	3-	- 187.5				Ċ	⊃51 →		0								
						-	-				/	/										
	5.0 185.4 Borehole Terminated at 5.0 m	SS	6	65	22	4.5 -	- 186		22	٢	/		g									
	GLI CONSULTANTS -			-			n compl	etion	of drillir	ıg: Dry	· · ·	\smile				-	noval:	Open	I			
	arrie, Ontario L4N 0B7 T : (705) 719-7994 Borehole details	ndwat	ed do n	ot cons	stitute a	thorough	understa	inding	of all poter	ntial con	ditions pr	resent a	Ground	re interp	retative	assistar	ice from			Scale	:1:75	
w	/ww.geiconsultants.com	chnical (enginee	er. Also,	, boreho	ole inform	ation sho	uld be i	ead in co	njunctio	n with the	geotec	hnical re	port for	which it	was					:1:/5 :1 of 1	



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Review
Local Benchmark:		

Drilling Method:	Hollow Stem Au	ugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821245	Date Started:	Apr 27/23
Reviewed By:	GW	Easting:	595296.6	Date Completed:	Apr 27/23

	LITHOLOGY PROFILE	SO	LSA	MPL	ING			FIEL	D TES	TING	;		LAE	B TES	TING			_			
								Shear St	ength Te										MMOC 8		5
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	4 (m)	ELEVATION (m)	 X Other Te + Pocket P ▲ Field Var △ Field Var 40 	enetrome e (Intact)		60		Combust Fotal Org 00 2	ible Organ ible Organ anic Vap 200 3 erberg Lin	nic Vapo our (ppm 00 4	ur (%LEL)	Instrumentation Installation		GRAIN STRIE (%	I SIZI BUTIO	
itholo		ample	ample	Recove	PT "N	DEPTH (m)	ILEVA	O SPT	tration Te	PT		PL H		r Content		— Ц	nstrum nstalla	GR	SA	SI	CL
	TILL. Clayey sill, some sand, inth,	-	1	80	6	0	-	0 6	20 3	<u>10</u>	10				30	40					<u> </u>
	0.8 brown, moist 189.3 CLAYEY SILT: Some sand, very stiff, brown, moist	SS	2	100	16	-	- 189	16 9					18 C								
	Trace gravel	SS	3	100	19	1.5 —	-	\ 19 \	2				12 O								
	2.3 187.8					-															
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, dense to very dense, brown,	SS	4	100	45		- 187.5				450_	8									
	moist	SS	5	100	84	3-					084 →	1	0								
		- 33	5	100	04		-				004 -] `	Ĭ								
						-	- 186														
						4.5 -	100														
		SS	6	100	35	4.5	-			350	/		12 O								
CH ESI	5.0 185.0 Borehole Terminated at 5.0 m																				
	SEI CONSULTANTS -	ndwat	er de	pth er	ncount	tered or	n compl	etion of drilli	ng: Dry		\bigcirc	Cave d	epth a	fter au	ger rei	moval:	Open				
647	Wolhom Road Unit 14	ndwat	er de	pth ob	oserve	ed on:					(Ground	lwater	Elevat	ion:						
w	T : (705) 719-7994 ww.geiconsultants.com Borehole details a qualified geotec commissioned ar	hnical e	enginee	er. Also,	, boreho	ole inform	ation sho	uld be read in co	ntial con njunctio	ditions n with th	oresent a le geote	and requi	ire interp eport for	oretative which it	assistar was	nce from			Scale:		



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	C
Project Location:	6728 Sixth Line, Milton, ON	L
Drilling Location:	See Borehole Location Plan	F
Local Benchmark:		

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821021	Date Started:	Apr 27/23
Reviewed By:	GW	Easting:	595221.8	Date Completed:	Apr 27/23

LITHOLOGY PROFILE SOIL SAMPLING Shear Strength Testing (kPa)	COMMENTS
Image: Sinear Streight Testing (kra) △ Combustible Organic Vapour (ppm) DESCRIPTION a a a a b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b b c b b b b b b c b b b b c b c b b b c c c c b b c c c c c b c c c c c c c c c c c c c<	& GRAIN SIZE DISTRIBUTION (%)
0 1 1 0 0 0 1 1 1 0 0 0 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 </td <td>GR SA SI CL</td>	GR SA SI CL
0.2 TOPSOIL: 150 mm 191.7 FILL: Clayey silt, Some sand, trace SS 1 80 7 0 0	
0.8 organics, firm, brown, moist 191.1 CLAYEY SILT: Some sand, trace gravel, very stiff, brown, moist SS 2 100 18	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4 17 47 32
SILT AND SAND: Trace clay, compact, Image: Marcine clay and the second sec	First Water Strike SS4
brown, wet	
SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact, brownish grey, moist	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	
SS 8 100 19 190 11	
SS 8 100 19 19 0 0	
Borehole Terminated at 8.1 m	
GEL CONSULTANTS	
GEI CONSULTANTS Groundwater depth encountered on completion of drilling: 7.3 m. Cave depth after auger removal: Open G47 Welham Road, Unit 14 Barrie, Ontario L4N 087 Groundwater depth observed on: Groundwater Elevation:	



Project Number:	2300805	_
Project Client:	Anatolia	_
Project Name:	Proposed Commercial Warehouse	Drilling Method
Project Location:	6728 Sixth Line, Milton, ON	Logged By:
Drilling Location:	See Borehole Location Plan	Reviewed By:
Local Benchmark:		

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821109	Date Started:	Apr 27/23
Reviewed By:	GW	Easting:	595294.3	Date Completed:	Apr 27/23

	LITHOLOGY PROFILE	SOIL SAMPLING						FIELD TESTII Shear Strength Testing									COMMENT				s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× +	Other Te Pocket P Field Var Field Var 40	st enetrometer le (Intact) le (Remolded) 30 120	kPa) 160		Combustil Combustil Total Orga 100 20 Atter	ble Orgai	nic Vapou our (ppm 00 4	ur (%LEL)) 00	Instrumentation Installation	G	8 RAIN STRIE	& N SIZI	E
Litholo	0.0 189.9	Samp	Samp	Recov	SPT "	DEPT	ELEV	0		DCPT 20	40) Water	Content	(%)		Instru Install	GR	SA	SI	CL
	0.2 TOPSOIL: 205 mm 1893 P.2 TOPSOIL: 205 mm 1893 FILL: Clayey silt, some sand, trace 0.6 gravel, firm, brown, moist 1893	ss	1	65	5	0	-	Q 5	<u>.</u>	20 30	40		0 2			+0			<u> </u>		
	CLAYEY SILT: Some sand, trace gravel, very stiff, brown, moist	ss	2	100	19	-	- 189		190				16								
	1.5 188.4				-					` \											
	SANDY SILT GLACIAL TILL: Trace gravel, trace clay, inferred cobbles and		3	100	37	1.5 —	-			37 0	2		12 O								
	boulders, dense to compact, brown to					_				i											
	brownish grey, moist	SS	4	100	35		- 187.5			350			11 P								
						3-															
		SS	5	100	21	0	-		21	¢			2								
						-				1											
							- 186														
						4.5 -				ļ											
2.00 000 000 000 000 000 000 000 000 000	5.0 184.9	ss	6	100	19		-		19 🤇	5			1 0 								
	Borehole Terminated at 5.0 m																			-	
										_											
	GELCONSULTANTS						ו compl	etion	of drilli	ng: Dry	\sim		lepth af			noval:	Open				
	arrie, Ontario L4N 0B7	ndwat							f all = 1				dwater								
w	T: (705) 719-7994 ww.geiconsultants.com a qualified geotec commissioned an	chnical e	enginee	er. Also,	boreho	le inform	ation sho	uld be r	ead in co	inial condition	the geote	anu requ echnical r	eport for	which it	was	ice irom				:1 :75 :1 of 1	

F		OF BOREHO	LE	No	. 13	3														K	$\overline{)}$	
Ρ	roject Number:	2300805																G	FI		Ľ	
	roject Client:	Anatolia																U		Cons	ultant	S
	roject Name:	Proposed Commerce			ouse				-		Solid S					Drilling M	-					
	roject Location: rilling Location:	6728 Sixth Line, Mil							jed By: ewed E			DP W		rthing: sting:		482120		Date St Date Co			Apr 26 Apr 26	
	ocal Benchmark:	<u>Dee Borenole Locat</u>		lan				T(C VI		Jy				sung.		000001		Date Ot	Simplet	cu. <u> </u>	-pi 20	/25
									<u> </u>					i			-					
	LITHOLOG	LITHOLOGY PROFILE SOIL SAMPLING									D TES rength Te				LAB	TESTIN	G		С	омм		6
ot			Q	nber	(%	Ine		(m) M	+ F		st enetrome ne (Intact)	ter		🔺 0	combusti	ble Organic Va ble Organic Va anic Vapour (p 00 300	apour (%LEL	tion		& RAIN STRIB	SIZE	
gy Pl	DESC	RIPTION	e Typ	e Nur	ery (%	N" Val	(m) H	ATIO		0		20 1	<u>6</u> 0			rberg Limits		nenta ation		(%	6)	
Lithology Plot		100.3	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	0	SPT	etration Te DCI 20 3	PT	0			Content (%)	LL	Instrumentation Installation	GR	SA	SI	CL
$\overline{\otimes}$		OIL: 50 mm	ss		75	6				<u> </u>	20 3	4	10	1	0 2		40					
\bigotimes		yey silt, firm, brown, ry moist _{189.5}	- 33	1	/5	0		-	0 6 \							0 27						
Ĭ		Some sand, very stiff, vn, moist	SS	2	100	17	-			179					18 O			<u>.</u>	First V	Vater S	Strike	SS2
							1.5 —	- 189	<u> </u>	١					18 O							
			SS	3	100	20		-		20					0							
			ss	4	100	27	-				27 0				14 O							
	3.0	187.2					3-	- 187.5				<u>\</u>										
	clay, trace gravel	LACIAL TILL: Some , inferred cobbles and	ss	5	100	48							480	1	0			\bigotimes				
		to compact, brown to h grey, moist					-	-				/	/					\mathbb{K}				
								- 186	5									\bigotimes				
			ss	6	100	23	4.5 —			2	30 30				11			\bigotimes				
悄も	5.0 Borehole Tei	185.3 rminated at 5.0 m				20												KX/				
	GEI CONSULTAN	10	ndwat	er de	pth en	icount	ered or	n comp	letion o	of drilli	ng: Dry	, 	Ċ	ave de	epth af	fter auger	removal:	Open				
64	7 Welham Road, U	Jnit 14 🛛 🗶 Groun	ndwat	er de	pth ob	serve	d on:M	ay 23/2	23 at de	epth o	f: 1.0	m.	G	Ground	water	Elevation:	189.3 m					

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Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'.



F		OF BOREHO	LE	No	. 14	ŀ																\bigcirc	
Ρ	roject Number:	2300805																	G	F			
	roject Client:	Anatolia																	U		Con	sultant	S
	roject Name:	Proposed Commerce 6728 Sixth Line, Mili			ouse				-			tem A DP	Augers				ig Mac 21572		Track M			May 4	/22
	roject Location: rilling Location:	See Borehole Locat							ed By: ewed E			W		rthing: sting:			5174.6		Date St Date Co			May 1 May 1	
	ocal Benchmark:	<u>See Borenole Locat</u>		lan				T C VI		у	U		La	sung.			/1/ 4.0	<u> </u>	Date Of	ompic		iviay i	/20
									1				_										
	LITHOLOG	Y PROFILE	SOI	L SA	MPL	ING					D TES				LAE	B TES	TING	5		(COMN		S
Plot	DESC		ype	Jumber	(%)	Value	(E	(m) NOI	+ P ▲ F △ F	ield Van ield Van	enetrome le (Intact) le (Remo	ded)		▲ 0	ombust otal Org	ible Orga Janic Vap 200 3	nic Vapo our (ppr 300	our (ppm) our (%LEI m) 400			ة GRAIN STRIE ("	N SIZE	
Lithology Plot	0.0	190.0	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	0 s	Pene PT	tration Te	esting PT	1 <u>60</u> 40		Wate	r Conten		Ц	Instrumentation Installation	GR	SA	si	CL
	CLAYEY SILT	DIL: 175 mm ^{189.9} : Some sand, trace /ery stiff, grey, moist	ss	1	75	6	0	-	0 6						(1						•		
	graver, min to v	ery suit, grey, moist	SS	2	100	17	-	- 189		\ 17γ					18 C	3							
			ss	3	100	20	1.5 —	_		20 (<u> </u>				14 O								
	2.3	187.8																					
	clay, trace gravel	LACIAL TILL: Some , inferred cobbles and	SS	4	100	27		- 187.5			27 à			9									
		act to dense, brown, moist	SS	5	100	48	3-	-					48)0		1 1 0								
20-00-00-							4.5 —	- 186			/	/			11		S						
		185.0	SS	6	100	23		-		2	30												
	Borehole Te	rminated at 5.0 m																					
64	GEI CONSULTAN 7 Welham Road, I	Jnit 14 T Groun					tered or d on:M											emoval: 89.5 m					

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Scale: **1 :75**

Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'.

Scale: 1 :75 Page: 1 of 1



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling Metho
Project Location:	6728 Sixth Line, Milton, ON	Logged By:
Drilling Location:	See Borehole Location Plan	Reviewed By:
Local Benchmark:		

Drilling Method:	Hollow Stem A	lugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821605	Date Started:	Apr 21/23
Reviewed By:	GW	_ Easting:	595261.4	Date Completed:	Apr 21/23q

	LITHOLOGY PROFILE	LITHOLOGY PROFILE SOIL SAMPLIN			ING	NG FIELD TESTIN Shear Strength Testing											COMMENTS				
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)		Other Tes Pocket Pe Field Van Field Van O 8 Pene SPT	t netrometer	160		Combust Total Org 00 2 Atte	anic Vapo 200 3 erberg Lim r Content	nic Vapou our (ppm) 00 4 nits (%)	ur (%LEL)	Instrumentation Installation	G	& RAIN STRIB (%	SIZ SUTIC	E
	8.0 188.2 ↓ TOPSOIL: 75 mm FILL: Sand and silt, trace clay, trace organics, trace gravel, loose, brown 188.5 0.0000000000000000000000000000000000	ss	1	100	5	0	- 189	9 5		0 30	40		0 17			+0					
	CLAYEY SILT: Some sand, trace gravel, till-like, inferred cobbles and	ss	2	100	10	-	-	10	R				12 O								
	boulders, stiff to very stiff, brown, moist	SS	3	90	16	1.5 —	- 187.5		16 V				2	20							
	2.3 187.0 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and	SS	4	90	24	-	-		2	40 40		1									
	boulders, compact to dense, brown to brownish grey, moist	SS	5	100	32	3 —	- 186			320		6									
00000000000000000000000000000000000000						-	-														
	5.0 184.2	ss	6	100	15	4.5 —	— 184.5		, 15ර			1									
	Borehole Terminated at 5.0 m																				
	7 Welbam Road Init 14				ncount oserve		n compl	letion c	of drillin	ıg: Dry		Cave d Ground			-	noval: (Open				
В	arrie, Ontario L4N 0B7 T : (705) 719-7994 Borehole details p	presente chnical e	ed do n enginee	ot cons er. Also	titute a , boreho	thorough ble inform	ation sho	Inderstanding of all potential conditions present and require interpretative assistance from Scale:1:75													



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Review
Local Benchmark:		

Drilling Method:	Solid Stem Aug	jers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821700	Date Started:	Mar 9/23
Reviewed By:	GW	Easting:	595280.3	Date Completed:	Mar 9/23

		0.01				<u> </u>		1											
	LITHOLOGY PROFILE	SOIL SAMPLING							FIELD TESTING Shear Strength Testing (kP		LAE	TEST	ING			c	OMM		S
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× + + +	Other Test Pocket Penetrometer Field Vane (Intact) Field Vane (Remolded)	60		ble Organ	iic Vapou our (ppm) 0 40	r (%LEL)	Instrumentation Installation			I SIZ BUTI(6)	
Litho	0.0 189,6	Sam	Sam	Rec	SPT		ELE	0		40		Content (20 31		0	Instr Insta	GR	SA	SI	CL
	0.2 TOPSOIL: 205 mm 189.4 SAND AND SILT: Some clay, loose to compact, brown, moist		1	75	5	0	- 189	05				0 23							
	1.5 188.1	SS	2	100	13			1	30		16 O								
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact to dense, brown,	00	3	100	26	1.5 -	- 187.5		260		14								
	moist to wet	SS	4	100	24				240		13 O					First V	Vater \$	Strike	SS4
00000000000000000000000000000000000000		SS	5	100	42	3-	- 186		42	20	10								
							- 100												
	5.0 184.6 Borehole Terminated at 5.0 m	SS	6	100	27	4.5-			27 ර		9 C								
647	Welham Road, Unit 14					tered o ed on:	n compl	letion	of drilling: Dry	<u> </u>	Cave depth a Groundwater			noval: (Open				
	ww.geiconsultants.com a qualified geoted	chnical of	enginee	er. Also	, borehe	te a thorough understanding of all potential conditions present and require interpretative assistance from Scale: 1:75 rehole information should be read in conjunction with the geotechnical report for which it was page: 1 of 1													



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling M
Project Location:	6728 Sixth Line, Milton, ON	Logged B
Drilling Location:	See Borehole Location Plan	Reviewed
Local Benchmark:		
Leea. Denominant.		

Drilling Method:	Hollow Stem Au	ugers	Drilling Machine:	Track Mount		
Logged By:	SDP	Northing:	4821506	Date Started:	Apr 21/23	
Reviewed By:	GW	Easting:	595248.9	Date Completed:	Apr 21/23	

LITHOLOGY PROFIL	LE S	SOIL	SAM	IPĻ	NG				FIELD TESTING LAB TESTING Shear Strength Testing (kPa)					C	COMMENTS					
DESCRIPTION	189.9	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	X Oti + Po ▲ Fie	her Test sket Penetrometer Id Vane (Intact) Id Vane (Remolded) <u>80</u> 120 Penetration Testing	160 40		otal Orga 00 20 Atter Water	ble Orgar anic Vapo 20 30 rberg Lim Content	nic Vapou our (ppm) 00 4 nits (%)	ur (%LEL)	Instrumentation Installation	Ģ	8 RAIN STRIE (%	k I SIZ BUTI(E
FILL: Clayey silt, some sa organics, firm, dark brown	nd, trace n, moist			60	5	0	-	0 5		40		0 2	0 23							1
CLAYEY SILT: Some sand, s moist	stiff, brown,	SS	2 9	90	14	-	- 189	14	φ			17 0								
	:	SS	3 9	95	15	1.5 —	-	15	1 10 10			15 O								
FILL: Clayey silt, some sa organics, firm, dark brown 0.8 CLAYEY SILT: Some sand, s moist 2.3 SANDY SILT GLACIAL TIL clay, trace gravel, inferred co boulders, compact, brown to grey, wet to moist	obbles and o brownish	SS	4 9	95	23		- 187.5		23 2			13 O					First V	Vater S	Strike	SS4
grey, wet to moist		SS	5 1	100	29	3-	_		29		9 C									
						_	- 186													
Borehole Terminated at	184.8	SS	6 1	100	18	4.5 —	-		18 ්		8 Ö									
	-						n compl	etion of	drilling: Dry						noval: (Open	I			
Barrie, Ontario L4N 0B7 T : (705) 719-7994 www.geiconsultants.com		esented on ical eng	do not o jineer. A	consti Also, I	itute a t boreho	thorough le inform	ation sho	uld be read	I potential condition I in conjunction with	s present a		re interp	retative a	assistan	ce from			Scale: Page:		



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method	: Hollow Stem A	ugers	Drilling Machine:	Solid Stem Augers	s
Logged By:	SDP	Northing:	4821646	Date Started:	May 1/23
Reviewed By:	GW	Easting:	595336.2	Date Completed:	May 1/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				FIELD TESTI		LAE	B TEST	ING			C	COMMENTS					
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	$\times +$	Shear Strength Testing Other Test Pocket Penetrometer Field Vane (Intact) Field Vane (Remolded) 0 120 Penetration Testing SPT ● DCPT) 160	Combusi	ible Organ ible Organ ganic Vapo 200 30 erberg Limi r Content (ic Vapou our (ppm) 00 40 its	ır (%LEL)	Instrumentation Installation	G	8 BRAIN STRIE (%	I SIZE BUTIC	Ξ			
	0.0 190.8 0.2 TOPSOIL: 230 mm 190.6 FILL: Clayey silt, some sand, firm to	ം ss	ഗ് 1	뿐 100	5 5	<u> </u>	H 190.5		<u>10 20 30</u>	40	10	20 3		0	u Li		6/1		02			
	stiff, brown, moist							5			0 12 15											
		SS	2	55	6	1.5 -	-	69 \														
	Some organics	SS	3	100	11	1.5	- 189	11	à		10											
\sim	2.3 188.5 CLAYEY SAND AND SILT GLACIAL TILL: Some gravel, inferred cobbles	SS	4	100	22		-		22		10 0					12	31	31	26			
	and boulders, compact to dense, brown, moist Wet		_	400	40	3-	-		10 0		15 0					First V	Vater S	Strike	SS5			
	wei	SS	5	100	18		- 187.5		180													
							-		Ň.													
		SS	6	100	35	4.5	7 - 186		35	<u></u>	8											
			-			-	-			/												
							-															
	Sand seam	SS	7	100	25	6-	- 184.5		25 🗸		70											
						<u> </u>																
						7.5 -	-		/ /													
	8.1 182.7	SS	8	100	19		- 183		19 ්		12 O											
	Borehole Terminated at 8.1 m																					
			er do			ared o		etion	of drilling: 4.5 m		Cave depth a	fter our			37 m							
647	' Welham Road, Unit 14 🖉 Grour						сопр	GUUII	л аншну. 4.5 П		Groundwater			ioval. (<i>.</i> , III.							
		hnical e	enginee	r. Also,	boreho	ole inform	ation sho	uld be r	all potential condition ad in conjunction wi					ce from			Scale: Page:					



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method	Solid Stem Aug	jers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821435	Date Started:	Apr 25/23
Reviewed By:	GW	Easting:	595345.4	Date Completed:	Apr 25/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING		FIELD TESTING LAB TESTING						COMMENTS					
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)		Penetration Testing	⊃a) 1 <u>60</u>	Combustible (Combustible (Combustible (Total Organic 100 200 Atterber PL	Drganic Vapo Vapour (ppm 300 4 g Limits	ur (%LEL)	Instrumentation Installation	C Di	8 RAIN STRIE (؟	& N SIZE BUTIC %)	E DN
Lith	0.0 189.6 0.2 TOPSOIL 150 mm 189.5		San	Rec	SPT	DEF			SPT DCPT 10 20 30	40	O Water Cor 10 20		40	Insti Insti	GR	SA	SI	CL
	FILL: Clayey silt, some sand, firm, brown, moist	ss	1	80	8		- 189	8 8			0 18							
	CLAYEY SILT: Trace gravel, very stiff, dark brown, moist	SS	2	100	16		-		162		19 C							
		SS	3	100	22	1.5 —	- 187.5		22		21							
	2.3 187.4 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact, brownish grey,	00	4	100	29		- 167.5		290		11 0							
	moist to wet	SS	5	100	19	3-	-		19 0		8				First V	Nater :	Strike	SS5
		- 33	5	100	13		- 186		19 4									
10000000000000000000000000000000000000						7	7											
	50 1046	SS	6	100	20	4.5-	F		20 0		9 C							
1-61 6-61	5.0 184.6 Borehole Terminated at 5.0 m	,																
		ndwat	ter de	pth er	ncoun	tered or	n compl	etion o	of drilling: 4.5 m.	Ċ	Cave depth after	auger rei	noval:	Open				
	arrie, Ontario L4N 0B7		ter de				underet	ndine	fall potential conditions		Groundwater Ele							
w	T: (705) 719-7994 ww.geiconsultants.com a qualified geotec commissioned ar	chnical of	enginee	r. Also	, boreho	ole inform	h understanding of all potential conditions present and require interpretative assistance from Scale:1:75 nation should be read in conjunction with the geotechnical report for which it was Borring Log'. Page:1 of 1											



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling Metho
Project Location:	6728 Sixth Line, Milton, ON	Logged By:
Drilling Location:	See Borehole Location Plan	Reviewed By
Local Benchmark:		

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821473	Date Started:	Apr 26/23
Reviewed By:	GW	Easting:	595427.7	Date Completed:	Apr 26/23

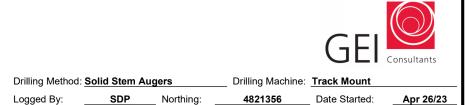
	LITHOLOGY PROFILE SOIL SAMPLING			FIELD TESTING Shear Strength Testing (kPa)			LAB TESTING				COMMENTS			s								
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)		Other Tes Pocket Pe Field Van Field Van 40 8 Pene SPT	e (Intact) e (Intact) e (Remol 0 1: tration Te DC	ter 20 16 Isting PT	<u>50</u>) Water	ble Organ anic Vapo 00 3 rberg Lim Content	nic Vapou our (ppm) 00 4 nits (%)	ur (%LEL)	Instrumentation Installation	6	8 GRAII STRII	& N SIZ	E
	8.9 188.5 ∖ TOPSOIL: 50 mm FILL: Sandy clayey silt, trace organics, firm, brown, moist	ss	1	80	5		- 189	0 5	10 2	20 3	04	0				30 4	10					1
	0.8 Inni, brown, most 188.7 CLAYEY SILT: Some sand, trace gravel, very stiff, brown, moist	SS	2	100	17	-	_		170					16 O					4	16	48	32
	1.5 188.0 SANDY SILT GLACIAL TILL: Some clay, trace gravel, cobbles and boulders, compact to dense, brown,	SS	3	100	17	1.5 —	- 187.5		17 ¢					12 O								
	moist	SS	4	100	22	-	-		22	α _	/			10								
	Sand seam, wet	SS	5	100	48	3-	- 186					48	о С	10					First	Water	Strike	SS5
						- 	Z			/		/										
	5.0 184.5 Borehole Terminated at 5.0 m	SS	6	100	21	4.5 -	 184.5		21	6				12 O								
64	7 Welham Road, Unit 14				icount oserve	tered or ed on:	n comp	letion	i of drillir	ng: 4.1	m	C	Cave c	lepth at dwater		-	noval:	Open	I			
	arrie, Ontario L4N 0B7 T : (705) 719-7994 ww.geiconsultants.com a qualified geote commissioned ar	oresente hnical e	ed do n enginee	ot cons er. Also,	titute a boreho	thorough ble inform	ation sho	uld be i					t and requ	ire interp	retative	assistan	ce from				:1:75	



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling I
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Reviewe
Local Benchmark:		

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821561	Date Started:	Apr 25/23
Reviewed By:	GW	Easting: _	595444.9	Date Completed: _	Apr 25/23

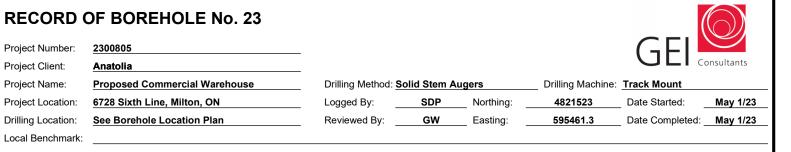
	LITHOLOGY PROFILE	SOIL SAMPLING						FIELD TE			LAB TES	TING		СОММЕ				
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	Shear Strength T X Other Test + Pocket Penetrom ▲ Field Vane (Intac △ Field Vane (Rem 40 80 Penetration T ○ SPT ● D	eter :) blded) 1 <u>20 160</u>	▲ Co ◇ To 100 PL ←	mbustible Orga tal Organic Var	3 <u>00 400</u> mits	nentation ation	GRA DISTR	& IN SIZE IBUTION (%)	L		
) HIHH	0.0 189.1 0.2 TOPSOIL: 150 mm 188.9		80 1	ଅ 85	ප් 4	<u></u>	H 189	<u>10 20</u>	<u>30 40</u>	10		<u>30 40</u>	sul					
	FILL: Clayey silt, some sand, firm, 0.8 brown, moist CLAYEY SILT: Some sand, trace				-	-	_	0 4			15							
	gravel, very stiff, brown, moist	SS	2	100	15			15 Q			16 O							
		SS	3	100	26	1.5 —	- 187.5	260			16 O							
	Sand seam, wet	SS	4	100	20		-	2 0 0		10				First Wate	r Strike SS	64		
	3.0 186.0		-	100	20	3-	- 186											
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, dense, brownish grey, moist	SS	5	100	31		100	31	, J									
						-	_		i I									
0.00 100 100						4.5 -	- 184.5				1		-					
		SS	6	100	34				34 ¢ 									
10000 0000						Ţ	Z											
		SS	7	100	30	6-	- 183	30	8	10								
				100	00	-	-		Ĭ,									
									N,									
2000 - 2000 2000 - 2000 - 2000 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 2000 - 200		SS	8	100	38	7.5 -	- 181.5		380	9 C								
RIE	8.1 181.0 Borehole Terminated at 8.1 m																	
	GEI CONSULTANTS	 ndwat	er de	pth en	l Icoun	tered or	n compl	etion of drilling: 5.	<u>; </u>	Cave de	oth after au	lger removal	Copen					
64	7 Welham Road, Unit 14 arrie, Ontario L4N 0B7						·				ater Eleva	-						
, <u> </u>																		



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821356	Date Started:	Apr 26/23
Reviewed By:	GW	Easting:	595430.7	Date Completed:	Apr 26/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING						STING			LA	B TES	TING			c	OMN		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)		Other Te Pocket F Field Va Field Va 40 Pen SPT	est Penetrome ne (Intact ne (Remo 80 1 etration T) Ided) I <u>20 1</u> esting CPT	60		Combus Total Or 100 Att	tible Orga ganic Vap 200 3 terberg Lin	nic Vapor our (ppm 00 4 nits (%)	ur (%I EI	Instrumentation Installation	G	8 RAIN STRIE	& N SIZ	Έ
	0.0 1883 └────────────────────────────────────		1	80	8		- 189	0	10	20	30 4	40		10 0 11	20 :	30 4	40					
	Trace gravel	SS	2	100	23	-	-		2	23 0			8									
	1.5 187.9 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact to very dense,		3	100	18	1.5 —	- 187.5		18 🤇					9								
	brown, moist	SS	4	100	43	-	-				4	3 Q		14 0								
184	Brownish grey	SS	5	100	52	3-	- 186					052 -	► C									
						4.5-	-				/	/										
	5.0 184.4 Borehole Terminated at 5.0 m	SS	6	100	24		 184.5			24 0				10				X				
647	Welham Road, Unit 14						n compl ay 23/2			0	· -	Ċ	Cave d Ground		after au r Elevat			l Open	<u> </u>			
	T : (705) 719-7994 www.geiconsultants.com Borehole details p a qualified geotec	Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'.																				



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial W
Project Location:	6728 Sixth Line, Milton,
D	

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING					STING		LAE	B TES	TING			COMMENTS		
Ethology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 X Other Ter + Pocket Per ▲ Field Van △ Field Van 40 	st enetrome e (Intact)	Ided) 20 160 esting	PL	Combusi Total Org 100 2 Atte	tible Organ ganic Vapo 200 3 erberg Lim	nic Vapo our (ppm 00 4	ur (%LEL		& GRAIN SIZE DISTRIBUTION (%)	CL	
	FILL: Clayey silt, some sand, firm,	o SS	0 1	90	5	0	ш — 189			30 40					40	드드			
	0.8 brown, moist to wet 188.5 CLAYEY SILT: Some sand, firm to very stiff, brown, moist		2	100	5	-	-	50 50					23 21			Ţ	First Water Strike SS	;2	
		SS	3	100	15	1.5 —	- 187.5	15 Q					27 0						
						-	-			280			14 0						
	3.0 186.3 SANDY SILT GLACIAL TILL: Some		4	100	28	3—	-		280										
	clay, inferred cobbles and boulders, dense to compact, brownish grey, moist	SS	5	60	39	-	- 186			39)0 /		14 0				•••			
						4.5 —	-		/										
		SS	6	100	26		- 184.5		26 ♀			11 0							
0-00-00-0-0-0-0-0-0-0-0-0-0-0-0-0-0-0-						-	-		 										
		SS	7	90	19	6	<u>/</u> 	19 🤇	{			12 O							
						_	-												
		SS	8	100	27	7.5 —	- 181.5		27 b			10							
8182	8.1 181.2 Borehole Terminated at 8.1 m						I												
	3FI CONSUL TANTS \\\\\ \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		er des	nth er	count	ered or	comp	etion of drilli	ng: 6 0		Cave o	lepth a	ffer au		moval	Open			
647	Y Welham Road, Unit 14 arrie, Ontario L4N 0B7	ndwat	er de	pth ob	serve	d on:M	ay 23/2	3 at depth of	f: 0.9	m	Ground	dwater	Elevat	on: 18	38.4 m				
w١	T : (705) 719-7994 Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was Scale: 1:75 WWW.geiconsultants.com Page: 1 of 1																		



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling Metho
Project Location:	6728 Sixth Line, Milton, ON	Logged By:
Drilling Location:	See Borehole Location Plan	Reviewed By:
Local Benchmark:		

Drilling Method:	Solid Stem Aug	jers	Drilling Machine	Buggy Mount	
Logged By:	SDP	Northing:	4821856	Date Started:	Mar 9/23
Reviewed By:	GW	Easting:	595377.8	_ Date Completed: _	Mar 9/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				FIELD					LAB	TES	TING				юмм	ENT	s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	×+▲⊲	Shear Stren Other Test Pocket Pene Field Vane (10 80 Penetra SPT	etromete Intact) Remold 12 tion Tes	er ed) 0 16			Combusti Fotal Orga 00 2 Atte	ble Orgar ble Orgar anic Vapo 00 30 rberg Lim	nic Vapou pur (ppm) 00 4 its	ur (%LEL)	Instrumentation Installation	6	8 RAIN STRIE (۶	L I SIZI BUTIO	E
	FILL: Sand and Silt, trace clay, loose,		0 1	₩ 45	6		ш — 189		0 20	30) 4	0					0					
	0.8 brown, wet 188.7 CLAYEY SILT: Some sand, stiff, brown wet	, ' SS	2	100	12	-	_	12	6						27			V	First \	Water \$	Strike	SS2
	1.5 187.9 CLAYEY SANDY SILT GLACIAL TILL: Trace gravel, inferred cobbles and boulders, compact, brown, moist	SS	3	100	20	1.5 —	- 187.5		20					13 O								
	Grey, wet	SS	4	100	14	-	-		40						9							
		SS	5	90	12	3 —	- 186	12	φ 1					13 O								
						_	-															
	5.0 184.4 Borehole Terminated at 5.0 m	SS	6	100	14	4.5	∠ ∠ — 184.5		40				1	0								
									i i i i i i i i i i i i i i i i i i i		_	\sim	Cave d					4.8 m.	1			
Ba	arrie, Ontario L4N 0B7 T : (705) 719-7994 www.geiconsultants.com	presente chnical e	ed do n enginee	ot cons er. Also,	titute a	thorough le inform	understa ation sho	nding o uld be r	epth of: (all potenti ad in conju	al cond	litions p	resent	Ground and requi	ire interp	retative	assistan				Scale	1 :75	1
	commissioned an								-											Page:	1 of 1	



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling N
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Reviewe
Local Benchmark:		

Drilling Method:	Solid Stem Au	igers	Drilling Machine:	Buggy Mount	
Logged By:	SDP	Northing:	4821795	Date Started:	Mar 12/23
Reviewed By:	GW	Easting:	595486.1	Date Completed:	Mar 12/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING			FIELD TESTING Shear Strength Testing (kPa)		LAB	TES	TING			с	OMN		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 Siteal Steingur Testing (kPa) X Other Test + Pocket Penetrometer ▲ Field Vane (Intact) △ Field Vane (Remolded) 40 80 120 160 Penetration Testing ○ SPT ● DCPT 	🔺 Co	ombustit otal Orga 0 20 Atter	ole Orgar		r (%LEL)	Instrumentation Installation		8 RAIN STRIE (۹ sa	I SIZ	
ŤŤ			ഗ് 1	_₩ 30	<u> の</u> 4	0	Ξ	10 20 30 40	10	2	03		0	드드				
	FILL: Silty sand, loose, brown, moist 0.8 188.8 SANDY CLAYEY SILT: Trace organics firm to stiff, brown, moist to wet 2.3 187.3 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact, brown, very moist 5.0 184.6 Borehole Terminated at 5.0 m	ss	2	100	5	-1	89	4 5 0		18	24 O							
	firm to stiff, brown, moist to wet			100	0	1.5 —				17					First V	Vator	Striko	553
	2.2 107.2	SS	3	100	14	₽	87.5	142		17 0					1 11 51 V	valer	SUIKE	000
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and	SS	4	100	18	_		18			2 9 C)						
	boulders, compact, brown, very moist	SS	5	100	14	3-		14 ¢		13 O								
							86											
2010 B						4.5 —				_								
	5.0 184.6 Borehole Terminated at 5.0 m	SS	6	100	23			23		1								
_		 	erde	oth er		ered on co	omol	letion of drilling: 1.8 m.	Cave de	nth af	ter aur	ner ren	noval· '	3.6 m				
64	GEI CONSULTANTS 7 Welham Road, Unit 14 arrie, Ontario L4N 0B7								Groundv	-								
	T : (705) 719-7994 Borehole details	hnical e	enginee	r. Also,	boreho	le informatio	n shou	anding of all potential conditions preser suld be read in conjunction with the geo y'.					ce from			Scale Page:		



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Review
Local Benchmark:		

Drilling Method:	Solid Stem Aug	jers	Drilling Machine:	Buggy Mount	
Logged By:	SDP	Northing:	4821869	Date Started:	Mar 9/23
Reviewed By:	GW	Easting:	595484.5	Date Completed:	Mar 9/23

LITHOLOGY PROFILE SOIL SAMPLING				ING			FIELD TEST	LA	B TESTING		COMMENTS			
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	Shear Strength Testing X Other Test + Pocket Penetrometer ▲ Field Vane (Intact) △ Field Vane (Remolded 40 80 120 Penetration Testin ○ SPT ● DCPT	160	Combus Total Or 100 Att PL	tible Organic Va tible Organic Va ganic Vapour (pr 200 300 erberg Limits er Content (%)	pour (%LEL)	Instrumentation Installation	& GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
	0.0 189.3 0.3 TOPSOIL: 280 mm 189.0 FILL: Sand and silt, loose, brown, moist		ย _ั 1	윤 100	_ds 4		H - 189	$\begin{array}{c c} 0 & 3P1 \\ \hline 10 & 20 & 30 \\ \hline 0 \\ 4 \end{array}$	40		20 30	40	sul sul	GR 3A 3I CL
			2	95	10	-	-	4 10 0			26 22 0		V	First Water Strike SS2
	moist	ss	3	100	20	1.5 —	- 187.5	200		14 0				
	2.3 187.0 SANDY SILT GLACIAL TILL: Some					-					,			
	clay, trace gravel, inferred cobbles and boulders, compact, brown, moist	SS	4	100	22	3-	-							
		SS	5	100	22	-	- 186	22 ¢		þ				
						4.5	-							
	5.0 184.3 Borehole Terminated at 5.0 m	SS	6	100	18	4.5 -	- 184.5	18 0		10				
64	7 Welham Road, Unit 14							letion of drilling: Dry 23 at depth of: 1.1 m.		Cave depth a			Open	
	T : (705) 719-7994 Borehole details p	oresente hnical e	ed do no	ot cons r. Also,	titute a	thorough le inform	understa ation sho	anding of all potential condition	ons present			ance from		Scale: 1 :75 Page: 1 of 1



Design the second	000005	
Project Number:	2300805	-
Project Client:	Anatolia	_
Project Name:	Proposed Commercial Warehouse	Drilling
Project Location:	6728 Sixth Line, Milton, ON	_ Logged
Drilling Location:	See Borehole Location Plan	Review
Local Benchmark:		

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821820	Date Started:	Mar 10/23
Reviewed By:	GW	Easting:	595579.4	Date Completed:	Mar 10/23

	LITHOLOGY PROFILE SOIL SAMPLING				FIELD TESTING Shear Strength Testing (kPa)					LAB TESTING					COMMENTS					
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 X Other Test + Pocket Per ▲ Field Vane △ Field Vane 40 80 	etrometer (Intact)		🔺 0	ombustit otal Orga 00 20	ole Orgai	our (ppm) 00 4	ur (%LEL)	Instrumentation Installation		8 GRAIN STRIE	& N SIZ	E
	0.0 187.2 ቢቶ ILL: Sand and gravel, loose, brovia	Sam	Sam	Reco	SPT	OEP			DCPT 30 40			Water 0 2			10	Instr Insta	GR	SA	SI	CL
ĬŬ	TOPSOIL: 180 mm		1	75	5		-	5					○ 23							
	SANDY CLAYEY SILT: Trace organics stiff, brown to grey, wet	SS	2	70	10		- 186	10 0				2	0				First \	Nater :	Strike	SS2
	1.5 185.7 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact, brown, moist		3	100	23	1.5 –	-	23	ې ۱			1 0								
		SS	4	100	20		- 184.5	20 🔆				1 0								
		SS	5	70	22	3-	- 104.5	22	5			1 0								
						-	- 183	/												
	4.6 182.6 SILT: Trace sand, compact, grey, wet					4.5 -	1	/					24 O							
	5.0 182.2 Borehole Terminated at 5.0 m	SS	6	100	12		-	12 ්					0							
		ndwat	er de	oth er	icount	tered or	n compl	etion of drilling	g: Dry _(\sum	L Cave de	epth af	ter au	ger rer	noval:	Open	1			
647	7 Welham Road, Unit 14 arrie, Ontario L4N 0B7	ndwat	er de	oth ob	serve	d on:					Ground	water I	Elevat	ion:						
	T : (705) 719-7994 Borehole details a qualified geotect	hnical e	enginee	r. Also,	boreho	a thorough understanding of all potential conditions present and require interpretative assistance from Scale: 1:75 ole information should be read in conjunction with the geotechnical report for which it was ation of Boring Log'.														



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Buggy Mount	
Logged By:	SDP	Northing:	4821729	Date Started:	Mar 10/23
Reviewed By:	GW	Easting:	595539.0	Date Completed:	Mar 10/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING						STING			LAB	TES	TING			6	OWN		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× + +	Other Te Pocket P Field Var Field Var 10 Pene		lded) 20 16 esting	a) 60		Combusti Total Orga 00 2 Atte	ble Orgar		r (%LEL)	Instrumentation Installation	c	8 RAIN STRIE	& N SIZ	E
	0.0 189.5 0.2 TOPSOIL: 125 mm 189.4 FILL: Silty sand, loose, brown, wet		ഗ് 1	2 45	4						30 4	0				i0 40	0	<u> </u>				
	Sand and silt	ss	2	100	4		- 189	4 04						14	27			₹ 	First \	Nater :	Strike	SS2
	1.5 188.0 SANDY CLAYEY SILT: Stiff, brown,	SS	3	100	10	1.5 -		10	<u> </u>						25 0							
	wet 2.3 SANDY SILT GLACIAL TILL: Some	:					- 187.5	10	Ň,					14								
	clay, trace gravel, inferred cobbles and boulders, compact, brown, wet to moist		4	100	18	3-	-		18¢					14 0								
0.00 000 000 000 000 000		SS	5	100	11		- 186	11	6					14 O								
0000000 000000000000000000000000000000						-	-		``	,												
000 000 000	5.0 184.5	SS	6	85	28	4.5 -	184.5			28 0			1	0								
							n compl lay 23/2				-	\sim	Cave d					Open	I <u></u>			
Ва	arrie, Ontario L4N 0B7 T : (705) 719-7994 ww.geiconsultants.com	oresente hnical e	ed do n enginee	ot cons er. Also,	titute a , boreho	thorough ble inform	understa ation sho	nding o uld be re	f all pote	ntial cor	ditions p	resent	and requi	re interp	retative	assistanc				Scale Page:		



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Review
Local Benchmark:		

Drilling Method:	Hollow Stem Au	ugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821552	Date Started:	Apr 21/23
Reviewed By:	GW	Easting:	595065.4	Date Completed:	May 1/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING					D TESTIN			LAB	TES	TING				COMMENTS		
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	×+ 4	Other To Pocket I Field Va Field Va 40	Penetrometer ne (Intact) ne (Remolded)	Pa) 160		Combustil otal Orga 00 20	ble Organ anic Vapo 00 3 rberg Lim	nic Vapor our (ppm 00 4	ur (ppm) ur (%LEL)) 00 	Instrumentation Installation	0	8 RAIN STRIE	& N SIZ	E
	<u>8.9 189.5</u> ∖TOPSOIL: 75 mm					<u> </u>	Ш		<u>10</u>		40		0 2		80 4	40	sul	GR	54	5	UL
	FILL: Clayey silt, some sand, firm, brown, moist	SS	1	90	6	-	_	6					0 14								
	CLAYEY SILT: Some sand, trace gravel, very stiff, brown, moist	SS	2	100	16				16 \				17 0								
		SS	3	100	15	1.5 —	- 189		150				14 O								
	Trace sand, wet					-	-							24 0				First \	Vater	Strike	SS4
	3.0 187.4	SS	4	100	15		- 107 5		15 Q					0							
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and	SS	5	100	27	3-	- 187.5			270			11 0								
	boulders, compact, brownish grey, very moist	r				-	-														
						4.5-	- 186		/												
		SS	6	65	11			1	10				0								
						-	_														
						6-	- 184.5						10								
		SS	7	100	11			1	10				1 2 O								
						-	-		``												
9 19 9 19 9 19 9 19 9 19						7.5 —	- 183		`			1	0								
놂쇖 힜녮	8.1 182.4 Borehole Terminated at 8.1 m	SS	8	100	19				19	5			0								
			orde			arad cr	Come	etion	ofdrill	ing: Dry				fter our		moval:	Onen				
647	GEI CONSULTANTS 7 Welham Road, Unit 14 arrie, Ontario L4N 0B7						Comp	euor		шу. ыу	\sim	Ground	-		-	noval:	Oheii				
	T : (705) 719-7994 ww.geiconsultants.com	hnical (enginee	r. Also,	boreho	ole inform	ation sho	uld be								ice from			Scale Page:		



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Reviewe
Local Benchmark:		

Drilling Method:	Hollow Stem Au	igers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821503	Date Started:	Apr 21/23
Reviewed By:	GW	Easting:	595121.7	Date Completed:	Apr 21/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING						STING sting (kPa			LAB	TES	TING			c	юмм		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× + +	Other Tes Pocket Pe Field Van Field Van	enetrome e (Intact)	ter (ded) 20 10	4) 60		Total Org 00 2	ble Orgar	nic Vapou pur (ppm) 00 4	ur (%LEL)	Instrumentation Installation	G	8 BRAIN STRIE	k N SIZI	E
Litho	8.9 191.8	Sam	Sam	Recc	SPT	DEP	ELEY	0	SPT	• DC	PT	0	'c) Water 10 2			10	Instru Insta	GR	SA	SI	CL
	FILL: Silty sand, trace clay, loose, brown, very moist	ss	1	100	8	0	-	ې 8						0 15								
	CLAYEY SILT: Some sand, trace gravel, stiff to very stiff, brown, moist	ss	2	25	10	-	- 190.5	10	5					15 O								
		SS	3	100	19	1.5 —			19 ¢	>				16 0								
		SS	4	100	17	-	-		 17 ්					18 0	8							
	3.0 188.7 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and	66	5	100	35	3-	- 189				35 9			12 O								
	boulders, dense to compact, brownish grey, moist					-	-				/ /											
						4.5	187.5			/	/											
		SS	6	100	27		-			27 d												
							- 186				`\											
	Very dense	SS	7	50	50+	6-						50+		16 0								
						-	-				, /	/										
						7.5 -	- 184.5			/	/											
	8.1 183.7	SS	8	100	21		-		21	б					23 〇							
	Borehole Terminated at 8.1 m																					
		<u> </u>	_																			
647	7 Welham Road, Unit 14	ndwat ndwat					n compl	etion	ot drillir	ng: 4.4	m	\sim	Cave d Ground	-		-	noval: 4	4.8 m.				
	T : (705) 719-7994 Borehole details	presente	ed do n	ot cons	titute a	thorough	understa	nding o	f all pote	ntial con	ditions p	resent a	and requi	ire interp	retative	assistan	ce from			Scale:	1 :75	
W	ww.geiconsultants.com								.au iii 60			~ 900160	di fe	-port für	amonit	.143				Page.	1 of 1	



Project Number:	2300805	_
Project Client:	Anatolia	_
Project Name:	Proposed Commercial Warehouse	_ D
Project Location:	6728 Sixth Line, Milton, ON	_ Lo
Drilling Location:	See Borehole Location Plan	_ R

Drilling Method:	Hollow Stem Au	ugers	Drilling Machine:	Solid Stem Augers	S
_ogged By:	SDP	Northing:	4821438	Date Started:	Apr 25/23
Reviewed By:	GW	Easting:	595217.2	_ Date Completed: _	Apr 25/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				D TESTI			LAB	TES	TING			СС	омме	NTS	3
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 X Other Tes + Pocket Pe ▲ Field Vane △ Field Vane 40 8 Penet 	t inetrometer e (Intact) e (Remolded) 0 120 ration Testing	160			ble Orgar anic Vapo 00 30 rberg Lim	nic Vapou our (ppm) 00 40	r (%LEL)	Instrumentation Installation	GI DIS	& RAIN TRIBU (%)	SIZE JTIC)	E)N
	0.0 190.5 0.2 TOPSOIL: 150 mm ^{190.3}	San	San	Rec	SP		190.5	O SPT 10 2	DCPT 0 30	40 :) Water 10 2			0		GR	SA	SI	CL
	FILL: Clayey silt, some sand, firm, brown, moist	SS	1	100	8			8					0 25							
	CLAYEY SILT: Some sand, stiff to very stiff, brown, moist		2	25	10			100				16 0				T				
		SS	3	100	19	1.5 —	- 189	19 ¢	1											
		ss	4	100	17		-	17 d				18 0								
	3.0 187.5 CLAYEY SAND AND SILT GLACIAL			400	0.5	3 —	- 187.5				8									
	TILL: Trace gravel, inferred cobbles and boulders, compact to dense, brownish grey, moist	SS	5	100	35		-		35 ç /											
						4.5-	- 186		/											
		SS	6	100	27				27 0		8									
						-	-		Ì,											
	Very dense	SS	7	50	50+	6 —	- 184.5			50+		13 0					First W 5	ater St 35	rike S 34	SS7 26
						-	-			//										
						7.5 —	- 183						23							
	8.1 182.4 Borehole Terminated at 8.1 m	SS	8	100	21			21	o'		-		23 〇							
	Borenole reminated at 6.1 m																			
		ndwat	er de	oth en	icount	tered or	n compl	etion of drillin	ig: Dry		Cave d	epth af	ter au	ger ren	noval: (Open				
647	Wolhom Road Unit 14	ndwat	er de	oth ob	serve	ed on:M	ay 23/2	3 at depth of	:1.0 m.	_ ~	Ground	lwater	Elevati	on: 18	9.5 m					
	T : (705) 719-7994 ww.geiconsultants.com a qualified geotec commissioned an	hnical e	enginee	r. Also,	boreho	ole inform	ation sho	uld be read in co							ce from			Scale: 1 Page: 1		



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method	Solid Stem Aug	jers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821382	Date Started:	Apr 25/23
Reviewed By:	GW	Easting:	595290.3	Date Completed:	Apr 25/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING							TING			LA	3 TES	TING				СОМИ	IENT	s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	×+ ▲⊲	Other Pocket Field V Field V 40	Test t Penet /ane (li /ane (F 80	romete ntact) Remolde 120	ed)) 16			Combus Total Or 00	tible Orga	our (ppm 300 4	ur (%LEL)) 00	Instrumentation Installation	6	GRAI STRI	& N SIZ	E
itholo		Sampl	Sampl	Secov	SPT "	DEPT	ELEV	0	Pe SPT 10	enetrati	on Test DCP 30	т	•			r Conten		LL 40	nstruı nstall	GR	SA	SI	CL
	0.0 191.0 0.3 TOPSOIL: 330 mm 190.7 FILL: Silty sand, loose, brown, wet	ss	1	75	4		- 190.5	0 4	10	20	30	4	0			20	0 32	40			1		
		SS	2	75	4	-			1							9				First	Water	Strike	SS2
**	1.5 189.4		-	10			-																
	SANDY CLAYEY SILT: Trace gravel, very stiff, brown, moist	ss	3	40	21	1.5 —			2	210					12 O								
		SS	4	100	22		- 189			1 22 4					15								
		- 33	4	100	22		-				$\mathbf{X}_{\mathbf{x}}$												
	3.0 187.9 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, dense, brownish grey, moist	SS	5	100	33	3-	- 187.5				33	γ		6									
	to wet						_																
						4.5 -							`\		13								
		SS	6	20	45		- 186					4	150		13 〇								
						-						/	/										
2000 2000 2000						6-	-					/											
		SS	7	25	31		- 184.5				31	5			15 0								
2118 1981 1910						- 7	7			/													
						7.5-	-			/													
899.0 802.0	Silty clay layer	ss	8	100	10		- 183	10	¢ć						15 0								
171192	8.1 182.9 Borehole Terminated at 8.1 m						ļ																
		ndwat	er dep	oth en	coun	tered or	n compl	etior	of dri	lling:	7.0	m	\Box	Cave d	epth a	ifter au	iger rer	noval:	Open				
	arrie, Ontario L4N 0B7					ed on:										Eleva							
w	T : (705) 719-7994 ww.geiconsultants.com a qualified geotec commissioned an	hnical e	enginee	r. Also,	boreho	ole inform	ation sho	uld be										ice from				:1 :75 :1 of :	



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821304	Date Started:	Apr 26/23
Reviewed By:	GW	Easting:	595357.3	Date Completed:	Apr 26/23

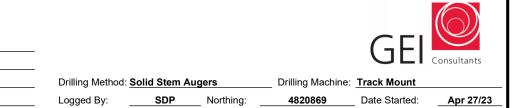
	LITHOLOGY PROFILE	SOI	L SA	MPL	ING			Γ	FIELD TESTI		LAE	B TEST	ING			СОМИ		
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	×+ ▲	Pocket Penetrometer Field Vane (Intact) Field Vane (Remolded) 40 80 120 Penetration Testin SPT DCPT) <u>160</u> g	Combusi Total Org 100 PL O Wate	ible Organi ible Organi ganic Vapo 200 30 erberg Limi	ic Vapou ur (ppm) 0 40 ts %)		Instrumentation Installation	GRAII DISTRI	& N SIZE	
	0.0 1906 0.2 TOPSOIL: 150 mm 190.4 FILL: Silty sand, loose, brown, moist	ss	1	80	4	0	- 190.5	0 4	10 20 30	40	10	20 30 0 21	<u> </u>	.0		I		
	0.8 189.8 CLAYEY SILT: Some sand, firm to very stiff, brown, moist	SS	2	100	6	-	-	6 d	২			20						
		SS	3	100	20	1.5 —	- 189	-	200		13 0							
	Wet	ss	4	100	18	-	-		18 6			ξ.				First Water	Strike S	SS4
	3.0 187.5		4	100	10	3—	- 187.5		18 9									
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact, brown, moist to wet	SS	5	95	22		107.0		22 ¢ \		12 O							
						-	-											
10-00 10-00		SS	6	100	27	4.5	Z 186		27 👌		10							
						-	-											
3 0 0 0 0 0 0 0						6-	- 184.5	_			11							
		SS	7	100	25	7	<i>(</i> -		25 ¢ /									
							~											
	8.1 182.5	SS	8	100	14	7.5	- 183		14 0		12							
	Borehole Terminated at 8.1 m																	
		 ndwat	er dei	oth er		ered or	l compl		n of drilling: 4.5 m	n. (Cave depth a	Ifter aug	er ren	noval: 3	7.0 m			
647	Gour Groun Welham Road, Unit 14 Arrie, Ontario L4N 0B7										Groundwater							
	T: (705) 719-7994 Borehole details p	hnical e	enginee	r. Also,	boreho	le inform	ation sho	ould be	g of all potential conditio e read in conjunction wi					ce from			:1:75 :1 of 1	



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821268	Date Started:	Apr 26/23
Reviewed By:	GW	Easting:	595415.3	Date Completed:	Apr 26/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				TESTING			LAB TE	STING			c	омм	FNT	s
	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 X Other Tes + Pocket Pe ▲ Field Vana △ Field Vana 40 8 	netrometer (Intact) (Remolded) 0 120 1 ration Testing • DCPT	a) 60 10	l ▲ c		anic Vapo pour (ppm <u>300</u> 4 imits nt (%)	ur (%LEL)	Instrumentation Installation	G	& RAIN STRIB (%	SIZI	E
	0.0 187.2 0.1 TOPSOIL: 125 mm ^{187.1} FILL: Sandy silt, some clay, loose, brown, moist	ss	1	70	4	0	-		0 30 4			0 18							
	0.8 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4 166.4	00	2	100	19	-	- 186	190	,		1								
	brownish grey, moist	SS	3	100	31	1.5 —	-		310		9								
		SS	4	100	20	-	- 184.5	20 0	5		1	D							
		SS	5	100	21	3-		21	↓ }		1	<u>,</u>							
						-	- 183												
	Wet	SS	6	100	14	4.5 –		/ 14 <				1				First V	Vater S	Strike	SS6
						-	- 181.5		\										
		SS	7	100	27	6 —			27 0		9								
						Ţ	7 												
	8.1 179.1	SS	8	100	28	7.5 -	-		28		9								
	Borehole Terminated at 8.1 m																		
	GEI CONSULTANTS \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	ndwat	er dee	bth en	count	ered o	n compl	etion of drillir	a: 7.0 m.		Cave de	epth after a	uger rei	moval:	Open				
64	GEI CONSULTANTS 7 Welham Road, Unit 14 arrie, Ontario L4N 0B7									$\overline{\mathbf{\nabla}}$		water Eleva	-						
	T : (705) 719-7994 WWW.geiconsultants.com a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'. Scale: 1:75 Page: 1 of 1																		



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4820869	Date Started:	Apr 27/23
Reviewed By:	GW	Easting:	595098.0	Date Completed:	Apr 27/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING				FIELD TEST			LAB	TES	TING			c	омм	IENT	s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× + _	 Pocket Penetromete Field Vane (Intact) 	r ed) <u>160</u>	🔺 Co	mbustil tal Orga) 20	ble Organ ble Organ anic Vapo 00 30 berg Lim	nic Vapou pur (ppm) 00 40	ur (%LEL	Instrumentation Installation	Ģ	8 BRAIN STRIE	k N SIZ	E
Litho	8 .9 181.4	Sam	Sam	Rec	SPT			С	SPT DCP 10 20 30	г			Content 0 3	(%) 0 4	10		GR	SA	SI	CL
	CLAYEY SILT: Some sand, trace organics, firm to very stiff, brown, moist	SS	1	65	7	0	-		9				0 26			<u> </u>				
		SS	2	60	15		- 190.5		152				22 〇							
	1.5 189.8 SILT: Some clay, trace sand, compact, brown, wet		3	100	19	1.5 —	-		\ 19 \				24 O				First V	Vater \$	Strike	SS3
	2.3 189.1 SANDY SILT GLACIAL TILL: Some		4	90	22		- 189		22			1								
	clay, trace gravel, inferred cobbles and boulders, compact, brownish grey, wet					3-														
		SS	5	100	16	Ŭ	-		16 ¢							\square				
						-	- 187.5									\mathbb{K}				
									Ì							\bigotimes				
		SS	6	100	20	4.5 —	-		20 0			16 O				\bigotimes				
	5.0 186.3 Borehole Terminated at 5.0 m															$\wedge \vee /$				
	GEI CONSULTANTS	ndwat	i er de	ı pth er	l Icoun	L tered or	n compl	letio	n of drilling: Dry	Ċ	Cave de	oth af	ter auç	ger ren	noval:	Open	L			
64	7 Welham Road, Unit 14 arrie, Ontario L4N 0B7		-						t depth of: 0.4 n		Groundw									
w	T : (705) 719-7994 WWW.geiconsultants.com a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'. Scale: 1 :75 Page: 1 of 1																			



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling I
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Reviewe
Local Benchmark:		

Drilling Method:	Solid Stem Aug	jers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4820869	Date Started:	Apr 28/23
Reviewed By:	GW	Easting:	595240.4	Date Completed:	Apr 28/23

LITHOLOGY PROFILE SOIL SAMPLING								FIELD TESTING	LAB TESTING		COMMENTS
👯 🛔 Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	Shear Strength Testing (kPa) X Other Test + Pocket Penetrometer ▲ Field Vane (Intact) △ Field Vane (Remolded) 40 80 120 Penetration Testing ○ PPT		Instrumentation Installation	& GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
	FILL: Sandy silt, some clay, trace	ss	_ເ	₩ 100		<u> </u>	H 192	10 20 30 40	10 20 30 40		
	0.8 rootlets, loose, brown, moist to wet 191.6 SANDY CLAYEY SILT: Trace gravel,	SS	2	100	15			150			
	very stiff, brown, moist					1.5 -	-			-	
		SS	3	100	19	7	- 190.5 Z				
		SS	4	100	22		-	22			
	3.4 189.0 SANDY SILT GLACIAL TILL: Some	ss	5	100	16	3-	- 189	16 ¢	17		First Water Strike SS5
	clay, trace gravel, inferred cobbles and boulders, compact, brown, wet						-				
	4.8 187.5 5©RAVELLY SAND: Trace clay, tra664	SS	6	100	20	4.5 —	- 187.5	20 0	18		
	silt, dense, brownish grey, wet Borehole Terminated at 5.0 m										
647	Welham Road, Unit 14		-		ncount oserve		n compl		Cave depth after auger removal: Groundwater Elevation:	Open	
	T : (705) 719-7994 Www.geiconsultants.com a qualified geoted	05) 719-7994 Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from Scale: 1:75									

RECORD OF BO

F BOREHOLE No. 37						\bigcirc
2300805					(¬⊢	\smile
Anatolia						onsultants
Proposed Commercial Warehouse	Drilling Method:	Solid Stem A	ugers	Drilling Machine:	Track Mount	
6728 Sixth Line, Milton, ON	Logged By:	SDP	Northing:	4820989	Date Started:	Apr 28/23
See Borehole Location Plan	Reviewed By:	GW	Easting:	595256.3	Date Completed:	Apr 28/23

Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed
Project Location:	6728 Sixt
Drilling Location:	See Borel

Anatolia	
Proposed Commercial Warehouse	
6728 Sixth Line, Milton, ON	

	LITHOLOGY PROFILE	DGY PROFILE SOIL SAMPLING			ING			FIELD TESTING Shear Strength Testing (kPa)						LAB	TES	TING			COMMENTS			
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× +	Other Te Pocket P Field Var Field Var 40		er ded) 20 16			Combustil otal Orga 00 20	ble Orgar ble Orgar anic Vapo 00 30 t rberg Lim	iic Vapou our (ppm) 0 4	r (%LEL)	Instrumentation Installation		; GRAII STRI	& N SIZ	E
	0.0 192.0 0.2 TOPSOIL: 150 mm ^{191.8}					0 DEP	ELE			DCF 20 3	PT T	0	0	0 2	Content	(%) 0 4	0	Instr Insta	GR	SA	SI	CL
	FILL: Clayey silt, some sand, firm, brown, moist	SS	1	65	5		-	0 5							0 21							
	SANDY CLAYEY SILT: Trace gravel, inferred cobbles and boulders, compact to dense, brown, moist	SS	2	100	19		- 190.5		190					14 0				V				
		SS	3	100	21	1.5-	100.0		21	ά				15 O								
		SS	4	100	32		-			32	γ			12 O					3	22	49	26
	Wet	SS	5	100	30	3-	- 189			30 🤇	5				24 O				First	Water	Strike	SS5
						-	-			 												
	4.6 187.4 SANDY SILT GLACIAL TILL: Some					4.5 —	- 187.5							0								
	clay, trace gravel, cobbles and boulders, compact, brown, moist	SS	6	100	27		-			27 ¢ /				0								
							400			/												
0.000 		SS	7	100	21	6-	- 186		21	ģ			9 C									
						-	- Z			ĺ												
		SS	8	100	17	7.5 —	— 184.5		17 d					11 0								
	8.1 183.9 Borehole Terminated at 8.1 m	00	0	100	17				17 0													
	7 Wolhom Bood Unit 14					tered or	•			0	_	\sim	Cave de					Open				
В	arrie, Ontario L4N 0B7 T : (705) 719-7994 www.geiconsultants.com a qualified geotec	oundwater depth observed on: May 23/23 at depth of: 1.2 m. Groundwater Elevation: 190.8 m ils presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from occhnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was												Scale	:1 :75							
	commissioned an	d the ac	ccompa	nying 'E	Explana	ation of Bo	oring Log													Page	1 of 1	



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method:	Solid Stem Aug	ers	_ Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4820983	Date Started:	Apr 28/23
Reviewed By:	GW	Easting: _	595335.2	Date Completed:	Apr 28/23

	LITHOLOGY PROFILE	SOIL SAMPLING					FIELD TESTING Shear Strength Testing (kPa)				LAB TESTING						COMMENTS							
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	H (m)	DEPTH (m) ELEVATION (m)		DEPTH (m) ELEVATION (m)		X Other Test + Pocket Penetrometer ▲ Field Vane (Intact) ∆ Field Vane (Remolded) 40			▲	Combustible Organ Total Organic Vap 100 200 3 Atterberg Lim			Combustible Organic Vapour (%LEL) Total Organic Vapour (ppm) 100 200 300 400 Atterberg Limits		Instrumentation Installation	0	8 BRAIN STRIE	k N SIZI	E
Litholo	0.0 191.2	Sampl	Sampl	Recov	SPT "I		ELEV	0	SPT	ration Testi DCPT 0 30	ng 40	PL		r Content		LL 10	Instrur Install	GR	SA	SI	CL			
	0.0 191.2 0.2 TOPSOIL: 205 mm 191.0 CLAYEY SILT: Some sand, stiff, brown moist		1	90	11	0	- 190.5						C 18) 3										
	Some gravel	SS	2	100	19	-			19 ¢				14 O											
		SS	3	100	22	1.5 –			22	þ ļ			16 0											
	Trace gravel	SS	4	100	24	-	- 189		2	4d			15 ∳	+				3	18	47	32			
	Hard, wet	SS	5	100	44	3-					440		13 O					First \	Water \$	Strike	SS5			
						-	- 187.5																	
H	4.6 186.6 GRAVELLY SAND: Dense, brown, Wet					4.5-	ł				/	_	10											
	5.65ANDY SILT GLACIAL TILL: Traces.1	SS	6	100	32					32 🔿			Ö											
	gravel, inferred cobbles and boulders, dense, brownish grey, wet																							
	Borehole Terminated at 5.0 m																							
		ndwat	er dep	oth er	ncount	ered o	n compl	etion	of drillir	ng: 4.2	n	Cave	depth a	fter au	ger rer	noval:	Open	1						
64	7 Wolhom Rood Unit 14	ndwat	er dep	oth ob	serve	d on:						Grou	ndwater	Elevat	ion:									
T: (705) 719-7994 www.geiconsultants.com Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was											Scale: Page:													



Project Number:	2300805	_
Project Client:	Anatolia	_
Project Name:	Proposed Commercial Warehouse	_ Drilling Metho
Project Location:	6728 Sixth Line, Milton, ON	Logged By:
Drilling Location:	See Borehole Location Plan	Reviewed By:
Local Benchmark:		

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821080	Date Started:	Apr 28/23
Reviewed By:	GW	Easting:	595334.0	Date Completed:	Apr 28/23

	LITHOLOGY PROFILE	SOI	SOIL SAMPLING				FIELD TESTING Shear Strength Testing (kPa)				LAB TI		COMMENTS									
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 X Other + Pocl ▲ Field △ Field 40 	r Strength Testing i r Test et Penetrometer Vane (Intact) Vane (Remolded) <u>80 120</u> Penetration Testing	kPa) 160		Combustible (Combustible (Total Organic 00 200 Atterberg	Vapour (ppr 300	our (%LEL)	Instrumentation Installation	G	8 BRAIN STRIE (%	I SIZE BUTIC	Ξ			
Litho			Sam	Reco	SPT	DEP		O SPT 10	DCPT 20 30	40	'c	Water Cor 0 20	tent (%) 30	40	Instr Insta	GR	SA	SI	CL			
	0.2 TOPSOIL: 230 mm 190.3 CLAYEY SANDY SILT: Trace gravel, firm to very stiff, brown, moist	ss	1	80	7		_	7				0 18										
		SS	2	100	18	15-		5	1.5 - 189 -	190	1	89 1			14 O							
		SS	3	100	19	1.5-	105		96			15 O										
	2.3 188.2 SANDY SILT GLACIAL TILL: Some Clay, trace gravel, inferred cobbles and boulders, dense to compact, brown,		4	100	36	-			36	360												
中、1 2 2 3	moist	ss	5	100	44	3-	- 187.5			440	9											
						_	-															
100						4.5	- 186			/												
	5.0 185.4	ss	6	100	25	4.5	100		25 ර		1	0										
1-61 649	Borehole Terminated at 5.0 m	·																				
	7 Wolhom Bood Unit 14		-				n compl	etion of c	rilling: Dry	\sim		epth after		moval:	Open							
В	arrie, Ontario L4N 0B7 T : (705) 719-7994 Borehole details	presente	ed do n	ot cons	titute a	thorough			potential condition	s present	and requi		ive assista	nce from			Scale:	1 :75				
w	ww.geiconsultants.com a qualified geotec commissioned ar								n conjunction wit	the geote	chnical re	port for whi	ch it was				Page.					



Desired Newsberry	0000005	
Project Number:	2300805	_
Project Client:	Anatolia	_
Project Name:	Proposed Commercial Warehouse	_ D
Project Location:	6728 Sixth Line, Milton, ON	_ Lo
Drilling Location:	See Borehole Location Plan	_ R
Local Benchmark:		

Drilling Method:	Hollow Stem Au	igers	Drilling Machine:	Solid Stem Augers	s
ogged By:	SDP	Northing:	4821095	Date Started:	Apr 28/23
Reviewed By:	GW	Easting:	595423.2	Date Completed:	Apr 28/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING			FIELD TESTIN		LAB	TESTING			COMMENTS
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	Shear Strength Testing (kl × Other Test + Pocket Penetrometer ▲ Field Vane (Intact) △ Field Vane (Remolded) 40 80 120 Penetration Testing ○ SPT ● DCPT 10 20 30	1 <u>60</u> 40	Combusti	ble Organic Vapour ble Organic Vapour anic Vapour (ppm) 00 300 40 rberg Limits Content (%) 20 30 40	r (%LEL) 10 	Instrumentation Installation	& GRAIN SIZE DISTRIBUTION (%) GR SA SI CL
	0.2 TOPSOIL: 180 mm ^{190.0} FILL: Clayey silt, some sand, stiff, brown, very moist	ss	1	100	8	0	-		40		30 40	5		
	CLAYEY SILT: Some sand, stiff to very stiff, brownish grey, moist		2	100	14	-	- 189	14 0						
		SS	3	100	17	1.5 —	-	170		16 O				
	2.3 187.5 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and baulder gemeent to dense brown to	ss	4	100	12	-	- 187.5	120		14 0				
	boulders, compact to dense, brown to brownish grey, moist to very moist	SS	5	100	31	3 —	-	310		9 C			F	irst Water Strike SS5
0.0000 0.0000 0.0000						-	186							
	5.0 185.	SS	6	100	31	4.5 -		310		10				
	Borehole Terminated at 5.0 m													
64	GEI CONSULTANTS Groundwater depth encountered on completion of drilling: 4.2 m. Cave depth after auger removal: Open 47 Welham Road, Unit 14 Groundwater depth observed on: May 23/23 at depth of: 0.5 m. Groundwater Elevation: 189.7 m													
	T : (705) 719-7994 Borehole details a qualified geotec	Intario L4N 0B7 Groundwater deptil observed on may 25/25 at deptil of .0.5 m. Groundwater deptil observed on may 25/25 at deptil of .0.5 m. Groundwater deptil observed on may 25/25 at deptil of .0.5 m. 5) 719-7994 Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was Sca												Scale: 1 :75 Page: 1 of 1



Project Number:	2300805	_
Project Client:	Anatolia	_
Project Name:	Proposed Commercial Warehouse	Drilling N
Project Location:	6728 Sixth Line, Milton, ON	Logged
Drilling Location:	See Borehole Location Plan	Reviewe
Local Benchmark:		

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821162	Date Started:	Apr 28/23
Reviewed By:	GW	Easting:	595100.2	Date Completed:	Apr 28/23

	LITHOLOGY PROFILE	SO		MPL	ING					D TESTI			LAE	B TES	TING			6	OWN		s						
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	H (m)	H (m) ATION (m)		H (m) ATION (m)		rh (m) (ATION (m)		DEPTH (m) ELEVATION (m)		Other Te Pocket F Field Van Field Van 40	enetrometer ne (Intact) ne (Remolded) 80 120	160	\$	Total Org	ible Orga	nic Vapor our (ppm 00 4	ur (%IEL)	Instrumentation Installation	0	8 GRAIN STRIE	& N SIZI	E
Litholo	0.0 189.6	Sample	Sample	Recove	SPT "N	DEPTI	ELEV	0	SPT	etration Testing DCPT 20 30	40	PL) Wate			LL 40	Instrun Installa	GR	SA	sı	CL						
	^{0.2} TOPSOIL: 180 mm ^{189.6} FILL: Clayey sandy silt, trace gravel, firm, brown, moist _{189.6}	ss	1	90	5	0	-	0 5			40		0 13						1	1							
	SANDY CLAYEY SILT: Trace gravel, very stiff, brown, moist	ss	2	100	17	-	- 189		179				15 O														
	Some sand	SS	3	100	20	1.5 —	-		20	6			17 0														
	2.3 187.5				20		- 187.5		20																		
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, dense to very dense, brown	SS	4	100	31		- 187.5			310			10 0					7	30	46	17						
	to brownish grey, moist	SS	5	100	39	3-	-			3	9 Q	8	3														
						-	- 186					`\															
	5.0 184.6	ss	6	80	71	4.5 —	-				071	-	9														
PP4 450	Borehole Terminated at 5.0 m																										
	GELCONSULTANTS -						n compl	etior	of drilli	ng: Dry		Cave	-		-	noval:	Open										
	arrie, Ontario L4N 0B7						undare 4		of cll	ntial and 1147			dwater														
w	ww.deiconsultants.com a qualified geoted	chnical of	enginee	er. Also,	Groundwater depth observed on: Groundwater Elevation: Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'. Scale: 1 :75 Page: 1 of 1																						



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling N
Project Location:	6728 Sixth Line, Milton, ON	Logged E
Drilling Location:	See Borehole Location Plan	Reviewe
Local Benchmark:		

Drilling Method:	Solid Stem Aug	gers	Drilling Machine:	Track Mount		
Logged By:	SDP	Northing:	4821313	Date Started:	Apr 26/23	
Reviewed By:	GW	Easting:	595502.9	Date Completed:	Apr 26/23	

	LITHOLOGY PROFILE	SO	L SA	MPL	ING					D TES				LAE	TES	TING			c	OMN	IENT	S
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	×+ ▲	Other Te Pocket F Field Va Field Va		eter) Ided) I <u>20 1</u> esting	60 		Combust Total Org 00 2	ble Organ anic Vapo 00 3 rberg Lin	nic Vapo our (ppm 00 4 nits	ur (ppm) ur (%LEL)) 00	Instrumentation Installation	0	8 RAII STRII	& N SIZ BUTI(%)	E
ii H	0.0 188.6 0.1 TOPSOIL: 125 mm 188.5 CLAYEY SILT: Some sand, trace	ം SS	ഗ് 1	₩ 75	5 6	<u> </u>			<u>10</u>			40					40	<u>in</u>		0,1		
	gravel, firm to stiff, brown, moist	ss	2	100	9	-		9 (5						23 21 0							
	1.5 187.1					15-	- 187.5															
	SAND: Trace silt, trace gravel, trace clay, loose, brown, wet	SS	3	90	9	1.5 —	-	9	<u> </u>	/				16 O					7 First \	82 Vater	8 Strike	3 SS3
	2.3 186.4 SANDY SILT GLACIAL TILL: Some clay, trace gravel, cobbles and boulders, dense to compcat, brown,	SS	4	100	30	-	- 186			30	•			1 1 0								
	moist	SS	5	100	47	3-	-					470										
						-	— 184.5			/	, '	, X										
		SS	6	100	20	4.5	F		20	8				10								
1981	5.0 183.6 Borehole Terminated at 5.0 m				-		「															
	GEI CONSULTANTS Image: Second water depth encountered on completion of drilling: 4.5 m. Cave depth after auger removal: Open																					
	arrie, Ontario L4N 087				serve									lwater								
w	T : (705) 719-7994 ww.geiconsultants.com ww.geiconsultants.com	hnical e	enginee	r. Also,	boreho	le inform	ation sho	uldbeı									ice from				:1 :75 :1 of 1	



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Hollow Stem A	ugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821298	Date Started:	Mar 6/23
Reviewed By:	GW	Easting:	595546	Date Completed: _	May 1/23

	LITHOLOGY PROFILE SOIL SAMPLING				FIELD TESTING	LAB TESTING		COMMENTS					
Lithology Plot	DESCRIPTION	∍ Type	Sample Number	Recovery (%)	SPT "N" Value	4 (m)	ELEVATION (m)	Shear Strength Testing (kPa) X Other Test + Pocket Penetrometer ▲ Field Vane (Intact) △ Field Vane (Remolded) 40 80 120 160	Combustible Organic Vapour (ppm) Combustible Organic Vapour (%LEL Total Organic Vapour (ppm) 100 200 400 Atterberg Limits	Instrumentation Installation	GF	& RAIN SIZ TRIBUTI (%)	Έ
Litholo	0.0 185.8	Sample Type	Sample	Recove	SPT "N	DEPTH (m)	ELEVA	Penetration Testing ○ SPT ● DCPT 10 20 30 40	PL UL O Water Content (%) 10 20 30 40	Instrum Installa	GR	SA SI	CL
	0.2 TOPSOIL: 255 mm 185.5	SS	1	95	5	0	-	0	0 14				
	0.8 ISBN 185.0 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, dense to compact, brown,	00	2	90	34		- 184.5	349					
	moist	SS	3	55	39	1.5 -	-	390	10				
		SS	4	65	15		- 183	159	11 O				
		SS	5	100	19	3-	-	190					
						-	- 181.5						
	5.0 180.7 Borehole Terminated at 5.0 m	SS	6	100	18	4.5 -		18 0			6	34 44	16
64	GEI CONSULTANTS Groundwater depth encountered on completion of drilling: Dry Cave depth after auger removal: Open 647 Welham Road, Unit 14 Computedwater depth chapter depth												
	7 Welham Road, Unit 14 Image: Groundwater depth observed on: May 23/23 at depth of: -0.4 m. Groundwater Elevation: 186.2 m arrie, Ontario L4N 0B7 Image: Groundwater depth observed on: May 23/23 at depth of: -0.4 m. Groundwater Elevation: 186.2 m borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was Superior of the geotechnical report for which it was												



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821391	Date Started:	May 1/23
Reviewed By:	GW	Easting:	595492.5	Date Completed:	May 1/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				FIELD TE			LAB	TES	TING				сомм		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× + ▲	Pocket Penetrome Field Vane (Intact	eter I Ided) 20 160		Combusti Total Org 00 2	ble Organ ble Organ anic Vapo 00 3 rberg Lim	nic Vapou our (ppm 00 4	ır (%LEL)	Instrumentation Installation	6	8 GRAIN STRIE (%	L I SIZE BUTIC	E
Litho	00 186.1 186.0 186.0 186.0	Sam	Sam	Reco	SPT	DEP	H 186	0	SPT ● DC 10 20	PT 30 40			Content 20 3		1 10	Instr Insta	GR	SA	SI	CL
	FILL: Clayey silt, some sand, trace organics, firm, brown, wet 185.3	SS	1	75	7	-			7				0 24				First	Water S	Strike	SS2
	CLAYEY SILT: Some sand, trace organics, trace gravel, stiff to very stiff, brownish grey, wet	SS	2	100	8		-	8	6				9							
		SS	3	90	16	1.5 —	- 184.5		16 2			15 O								
		SS	4	100	20	-	-		20 0			14 0								
	3.1 Silt seam ^{183.0} SANDY SILT GLACIAL TILL: Some	ss	5	100	41	3-	- 183	-		41,0	8									
	clay, inferred cobbles and boulders, dense to compact, brown to brownish grey, moist					-	-													
0.8.0		SS	6	100	28	4.5 —	— 181.5	_	28 0			14 0								
	5.0 181.1 Borehole Terminated at 5.0 m	33	0	100	20				200											
	GEI CONSULTANTS -		-				n compl	letio	n of drilling: Dr	<u> </u>	Cave d	epth a	l fter au	ger rer	noval:	L Open	I			
	7 Welham Road, Unit 14 arrie, Ontario L4N 0B7 T : (705) 719-7994 Borehole details p						understa	anding) of all potential co	ditions preser	Ground and requi				ce from			Soale	4.75	
w	ww.geiconsultants.com	chnical e	enginee	r. Also,	boreho	le inform	ation sho	ould be	e read in conjunction	n with the geo	technical r	eport for	which it	was				Scale: Page:		



Project Number:	2300805	
Project Client:	Anatolia	
Project Name:	Proposed Commercial Warehouse	Drilling Method:
Project Location:	6728 Sixth Line, Milton, ON	Logged By:
Drilling Location:	See Borehole Location Plan	Reviewed By:
Local Benchmark:		

Drilling Method:	Hollow Stem Au	ugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821417	Date Started:	Mar 6/23
Reviewed By:	GW	Easting:	595587	Date Completed:	May 1/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING					D TES				LAE	B TES	TING				COMMENTS				
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× Ot + Po	her Tes ocket Pe eld Van eld Van Pene	ength Tes enetromet e (Intact) e (Remole 0 12 tration Te DCI	ter ded) 20 10 sting	a) 60		Combust Total Org 100 2 Atte	ible Orga	nits	ur (%LEL	Instrumentation Installation		ع GRAII STRII	& N SIZI	E		
	0.0 188.5 0.2 TOPSOIL: 205 mm 188.3 FILL: Clayey silt, trace sand, trace propriet to work		ഗ് 1	₩ 85	5	<u> </u>	<u> </u>	0 10 5				0			20 :		0	<u> </u>						
Ŵ	_{0.8} organics, firm, brown, moist to wet <u>187.7</u> CLAY AND SILT: Some sand, trace gravel, stiff to hard, greenish brown to	SS	2	100	13	-	- 187.5	13	6_	/ /				Ĥ	20	-			3	16	42	39		
	light brown, moist	SS	3	100	50+	1.5 —	-				<u> </u>	50+			21									
	2.3 186.2 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact to dense, brown,	60	4	100	27		- 186			27 9	/			12 O					First	Water	Strike	SS4		
00000000000000000000000000000000000000	moist	SS	5	100	31	3 —	-			31	b			12 O										
						-	— 184.5																	
000-00-00-00-00-00-00-00-00-00-00-00-00	5.0 183.5 Borehole Terminated at 5.0 m	SS	6	100	13	4.5	Z	13	6					11 0				Ŵ						
647	Welham Road, Unit 14							etion of 3 at de			-	\sim		-		ger rer ion: 18		u Open	1					
	Arrie, Ontario L4N 0B7 T : (705) 719-7994 ww.geiconsultants.com	oresente hnical e	ed do n enginee	ot cons r. Also,	titute a	thorough le inform	understa ation sho	nding of a uld be rea	all pote	ntial con	ditions p	resent	and requ	ire interp	oretative	assistan					:1 :75 1 of 1			



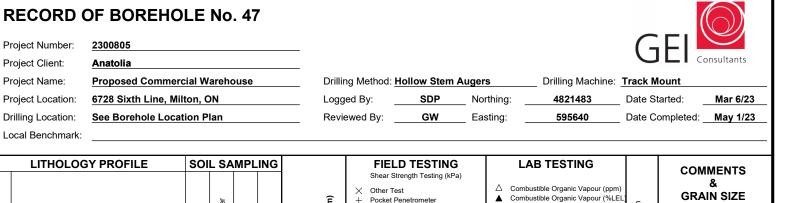
Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Hollow Stem A	ugers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821390	Date Started:	Mar 6/23
Reviewed By:	GW	Easting:	595644	Date Completed:	May 1/23
				· · · -	

	LITHOLOGY PROFILE	SOI	LSA	MPL	ING						TING			LAB	TES	TING			0	COMMENTS			
🗧 🚦 Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)		Other Te Pocket P Field Var Field Var		ded) 20 16			Combusti Fotal Org 00 2	ible Organ ible Organ anic Vap 2003 erberg Lin	nic Vapou our (ppm) 00 4	ur (%LEL)) 00	Instrumentation Installation	(8 GRAIN STRIE	& N SIZI	E	
Lithol	0.0 186.5	Samp	Samp	Reco	SPT -	DEPT	ELEV	0 5	SPT	DCI)			r Content 20 3		LL 10	Instru Instal	GR	SA	SI	CL	
	0.2 TOPSOIL: 230 mm 186.3 FILL: Clay, some sand, some silt, trace gravel, firm, brown 185.7	SS	1	85	7	0	- 186	27						0 17									
	0.2 IOPSOIL: 230 mm 186.3 FILL: Clay, some sand, some silt, trace gravel, firm, brown 185.7 CLAY AND SILT: Some sand, trace gravel, firm, greenish brown, light brown, moist 185.0 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact brown	SS	2	50	4	-	-	¢4							21 0								
	1.5 185.0 SANDY SILT GLACIAL TILL: Some	SS	3	100	10	1.5 —		10	<u>ې</u>					17 0									
	clay, trace gravel, inferred cobbles and boulders, compact, brown						- 184.5		Ì					10									
		SS	4	100	24		-		2	249				12 (]	+1				7	31	44	18	
		SS	5	100	27	3-				27 0				12 O									
							- 183			/													
	4.6. 494.0						<u> </u>		/	/													
	4.6 181.9 CLAYEY SANDY SILT GLACIAL TILL: 5.0Trace gravel, inferred cobbles and 1.4		6	100	15	4.5-	_ 181.5		150				ŀ	12 O					First 7	Vater 25	Strike 42	SS6 27	
1823	boulders, compact, grey, moist Borehole Terminated at 5.0 m						- 101.0																
		ndwat	er de	pth er	ncoun	tered or	n comp	etion o	of drilli	ng: Dry	<u> </u>	С	Cave d	epth a	fter au	ger rer	noval:	4.2 m.					
64	7 Welbam Road Linit 14	ndwat	er de	pth ob	oserve	ed on:						_	Ground	water	Elevat	ion:							
w	T: (705) 719-7994 ww.geiconsultants.com commissioned an	hnical e	enginee	er. Also,	, borehe	ole inform	ation sho	uld be re	f all pote ad in co	ential con onjunction	ditions p n with the	resent e geote	and requi chnical re	re interp eport for	vretative which it	assistan was	ice from				:1 :75 1 of 1		

Project Client:

Project Name:



gy Plot	DESCRIPTION	e Type	Sample Number	Recovery (%)	SPT "N" Value	(m)	ELEVATION (m)	 X Other Tes + Pocket Pe ▲ Field Vana △ Field Vana 40 8 	enetrome e (Intact)		0	▲ C ◇ T	otal Orga 00 20	ble Organ ble Organ anic Vapo 00 30 rberg Lim	iic Vapou our (ppm) 0 40	ur (%LEL)	Instrumentation Installation			, I SIZE BUTIC 6)	
Lithology Plot	0.0 188.3	Sample Type	Sample	Recove	SPT "N	DEPTH (m)	ELEVA	Penet O SPT 10 2	tration Te)	PL - 0 1		Content ((%)	- LL ю	Instrum Installa	GR	SA	SI	CL
	0.2 TOPSOIL: 230 mm 188.1 FILL: Clayey silt, trace sand, trace, firm, brown 187.5		1	85	5	0		0 5		1				0 29							
Ĥ	CLAY AND SILT: Some sand, trace gravel, stiff to firm, greenish brown, light brown, moist	SS	2	100	10	-	- 187.5	10 0						2 1 O							
		SS	3	100	6	1.5 -	-	60	•					28 〇				Vane s T = 10 arm le	0 lb-ft	at tore	
	2.3 186.0 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact to dense, brown,	00	4	100	21		- 186	21	Q					24 O					0		
10-01 000-00-0 000-00-00-00-00-00-00-00-00-00	moist	SS	5	100	33	3-	-		3	30		9 C									
							- 184.5														
- 0.00 S	Orange staining, darker brown-grey 	SS	6	100	16	4.5 -	-	16 ¢	Y			1	0								
						-	- 183														
		SS	7	100	14	6-	-	14 0					11 0					9	32	42	17
						1	181.5														
9-02-03-03 9-02-03-03		SS	8	100	23	7.5 -	-	2:	<u>з</u> ь				11 0								
	Borehole Terminated at 8.1 m																				
	7 Wolhom Bood Unit 14						n compl	letion of drillir	ng: 6.7	m	\smile		•			noval: (Open				
Ва	arrie, Ontario L4N 0B7 T : (705) 719-7994 www.geiconsultants.com a qualified geotec	oresente hnical e	ed do n enginee	ot cons r. Also,	titute a boreho	thorough ble inform	ation sho	uld be read in co			resent a	nd requi	re interp		assistan	ce from			Scale	1 :75	
	commissioned an	d the ad	ccompa	nying 'I	Explana	ation of B	oring Log	'.											Page:	1 of 1	



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	
	1

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821738	Date Started:	Mar 9/23
Reviewed By:	GW	Easting:	595220.3	Date Completed:	Mar 9/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				D TES			LAB	TES	TING			COMMENTS				
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 X Other T + Pocket ▲ Field Va △ Field Va 40 	Strength Test est Penetrome ane (Intact) ane (Remol 80 1: netration Te • DC	ter ded) 20 160 +		Combustil Total Orga 00 20 Atte	ble Organ ble Organ anic Vapc 00 30 rberg Lim Content	nic Vapou our (ppm) 00 4 its	ur (%LEL)	Instrumentation Installation	G	8 RAIN STRIE	k I SIZI	E	
	0.0 189.6 0.2 TOPSOIL: 255 mm 189.4 FILL: Sand and silt, very loose, brown,	ം ss	ഗ്ഗ് 1	<u>ළ</u> 100	ස් 1					<u>40</u>			0 3 0 27		10	sul	GIT	54	01	0L	
\sim	0.6 wet 189.0 SANDY CLAYEY SILT: Trace organics, stiff, brownish grey, wet	SS	2	100	12	-	- 189	1				18 0					First V	Vater \$	Strike	SS2	
	1.5 188.1 SANDY SILT GLACIAL TILL: Trace		2		12	1.5 —	-	\ \ \													
	gravel, trace clay, inferred cobbles and boudlers, compact, brown, moist	SS	3	100	16		- 187.5	16 Ċ				15 O									
10-10-10-10-10-10-10-10-10-10-10-10-10-1		SS	4	100	26				26 0			12 O									
900 (000) 1900 (000)		SS	5	100	27	3-			27 0			0									
20-10-0 20-0-0 20-0-0-0						-	- 186		/												
10.00 C						4.5 —	-					0									
10 10 10 10	5.0 184.6 Borehole Terminated at 5.0 m	SS	6	100	18			18				<u> </u>									
	SEI CONSULTANTS -	ndwat	er de	l pth en	icount	ered or	n compl	etion of dril	ing: Dry	<u> </u>	L Cave d	epth af	fter aug	ger rer	noval:	Open	I				
647	′ Welham Road, Unit 14 arrie, Ontario L4N 0B7 Grour										Ground										
w	ww.geiconsultants.com a qualified geotec	Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'. Page: 1 of 1																			



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Solid Stem Aug	jers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821767	Date Started:	Mar 9/23
Reviewed By:	GW	Easting:	595351.0	_ Date Completed: _	Mar 9/23

																	_				
	LITHOLOGY PROFILE	SOI	LSA	MPL	ING					D TEST			LAB	TES	TING			c	юмм	ENTS	5
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× 0 + F	Other Tes Pocket Pe Field Van Field Van		d) 160		Total Org	ble Orga	nic Vapo our (ppm 00 4	ur (%LEL	Instrumentation Installation		& BRAIN STRIB (%	SIZE UTIO	
Litho	0.0 188.4	Sam	Sam	Reco	SPT		ELEY	0 5	SPT	DCPT 0 30				Content		40	Instru Insta	GR	SA	SI	CL
	0.2 TOPSOIL: 255 mm 188.2 FILL: Sand and silt, loose, brown, wet		1	75	4	0	-	0 4						0 23							
	0.8 187.7 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and boulders, compact to dense, brown,	SS	2	100	13		- 187.5	1:	39				16 0					First \	Vater S	strike S	S2
	moist	SS	3	100	19	1.5 -	-		192				17 0								
		SS	4	100	31		- 186			310			12 O								
		SS	5	100	17	3-	-		17 Ý	/			9					7	32	45	16
						-	- 184.5		,												
		ss	6	100	24	4.5 -	}		2	40		·	10								
191 892	5.0 183.4 Borehole Terminated at 5.0 m																				
	GLICONJULIANIS	ndwat	er de	pth er	ncoun	tered or	n compl	etion o	: of drillir	ng: Dry	Ċ	Cave d	lepth a	fter au	ger rer	noval:	Open				
	7 Welham Road, Unit 14											Ground									
w	: (705) 719-7994 Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'. Scale: 1:7																				



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Buggy Mount	
Logged By:	SDP	Northing:	4821608	Date Started:	Mar 13/23
Reviewed By:	GW	Easting:	595565.0	Date Completed:	Mar 13/23

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING		Γ	FIELD TESTING		LAB	TES	TING				сомм		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m) ELEVATION (m)		Pocket Penetrometer Field Vane (Intact) Field Vane (Remolded) 40 80 120 160 Penetration Testing SPT • DCPT) Water	ble Organ anic Vapo 00 3 rberg Lim	nic Vapou our (ppm 00 4 nits (%)	ur (%LEL)) 00 	Instrumentation Installation	6	8 GRAIN STRIE (%	L I SIZE BUTIC	Ξ
	0.0 187.4 0.2 TOPSOIL: 150 mm 187.2 FILL: Sand and silt, orange brick waste, trace clay, loose to very loose, brown,	2	თ 1	₩ 75	<i>в</i>	<u>0</u>	Ę	<u>10 20 30 40</u>		10 2 C		80 4	10			<u> </u>		
	wet	SS	2	45	2	-	ر م					32 O			First	Water S	Strike	SS2
		66	3	100	18	1.5 — 186		180		12								
	clay, trace gravel, inferred cobbles and boulders, compact, brown to brownish grey, wet		3	100	10	-												
0 1 0	g.cy, not	SS	4	100	28	- 104 5		289		11 0								
		SS	5	75	27	3184.5		27 0		11 0								
						-												
						4.5				10								
	5.0 182.4 Borehole Terminated at 5.0 m	SS	6	95	15			150		12 O								
														1.9 m				
64	GEI CONSULTANTS 7 Welham Road, Unit 14 arrie, Ontario L4N 0B7						ietioi		Cave d Ground			-	noval: 4	4.8 m.				
	T : (705) 719-7994 Borehole details p a qualified geotec	Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from Scale: 1:75																



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method:	Solid Stem Aug	gers	Drilling Machine:	Buggy Mount	
Logged By:	SDP	Northing:	4821674	Date Started:	Mar 10/23
Reviewed By:	GW	Easting:	595597.8	Date Completed:	Mar 10/23
		_			

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING				FIELD TEST			LAB	TES	FING			COM	MENT	s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× +	Shear Strength Testir Dother Test Pocket Penetrometer Field Vane (Intact) Field Vane (Remolde 0 80 120 Penetration Testi	id) 160	▲ C	ombustil otal Orga 0 20	ble Orgar ble Orgar anic Vapo 20 30 rberg Lim	nic Vapou our (ppm 20 4	ur (%LEL)	Instrumentation Installation	GRA DISTR	& IN SIZ IBUTI((%)	E
Lithol	0.0 188.7	Samp	Samp	Reco	SPT '	DEP1	ELEV	0					Content			Instru Instal	GR SA	SI	CL
\bigotimes	0.2 TOPSOIL: 180 mm 188.5 FILL: Silty sand, trace organics, very 0.6 loose, brown, wet 188.1	SS	1	75	3	0	-	3					0 22						
	SANDY CLAYEY SILT: stiff, brown, we	ss	2	100	8	Ţ	<u>7</u> 	8¢					23 O				First Wate	r Strike	SS2
		SS	3	100	14	1.5 —	-		49				26 〇						
	2.3 186.4 SANDY SILT GLACIAL TILL: Trace gravel, trace clay, cobbles and boulders, compact, brownish grey, wet	SS	4	85	12	-	- 186	12	24			14 O							
	boulders, compact, brownish grey, wet	SS	5	100	12	3-	-	12	φ			13 O							
						-	<u> </u>												
56) G		SS	6	85	18	4.5 —			180			1							
	5.0 183.6 Borehole Terminated at 5.0 m						-												
		l ndwat	er de	pth en	licount	ered or	n compl	etion (of drilling: 0.9	m (Cave de	pth af	i ter auç	ger rer	noval: :	3.6 m.	<u> </u>		
647	′ Welham Road, Unit 14 arrie, Ontario L4N 0B7										Groundv								
wv	ww.geiconsultants.com a qualified geotec	Borehole details presented do not constitute a thorough understanding of all potential conditions present and require interpretative assistance from a qualified geotechnical engineer. Also, borehole information should be read in conjunction with the geotechnical report for which it was commissioned and the accompanying 'Explanation of Boring Log'. Page: 1 of 1																	



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan

Drilling Method:	Solid Stem Aug	jers	Drilling Machine:	Buggy Mount	
Logged By:	SDP	Northing:	4821642	Date Started:	Mar 13/23
Reviewed By:	GW	Easting:	595642.0	Date Completed:	Mar 13/23

	LITHOLOGY PROFILE	SO	L SA	MPL	ING			FIELD TESTING Shear Strength Testing (kPa)	LAE	B TESTING	NG			COMMENTS								
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	 Other Test Pocket Penetrometer Field Vane (Intact) △ Field Vane (Remolded) 40 80 120 160 Penetration Testing 	Combus Total Org 100 Att	tible Organic Vapor tible Organic Vapor ganic Vapour (ppm 200 300 4 erberg Limits	ur (%LEL)) 00	Instrumentation Installation		8 RAIN STRIE (%	SIZE							
Lithol	0.0 189.5	Samp	Samp	Reco	SPT '	DEPT	ELEV	O SPT ● DCPT 10 20 30 40	PL O Wate 10	er Content (%) 20 30 4	LL 40	Instru Instal	GR	SA	SI	CL						
	TOPSOIL: 50 mm FILL: Sand and silt, trace clay, trace organics, loose, brown, wet 189.1	SS	1	100	5	0	-	0 5		0 25												
	CLAY AND SILT: Trace sand, trace gravel, stiff to very stiff, brown, wet	SS	2	100	9	-	- 189	90		24 O			First V	Vater S	Strike S	SS2						
		SS	3	100	16	1.5 —	-	16		26			1	3	54	42						
		SS	4	100	12	-	- 187.5	120		8												
A REAL PROPERTY OF	3.0 186.6 SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and	SS	5	100	21	3 —	_	210	16 0													
	boulders, compact, brown to brownish grey, wet to moist					-	- 186					X										
		SS	6	100	12	4.5	7_	120	11			X										
i i i i i i	5.0 184.8 Borehole Terminated at 5.0 m	3		100								$\leq \sim /$										
		l ndwat	l er dep	oth en	licount	ered or	n compl	etion of drilling: 4.7 m.	Cave depth a	after auger rer	noval: 5	5.6 m.										
	arrie, Ontario L4N 087							3 at depth of:		ater Elevation:												
w	T : (705) 719-7994 ww.geiconsultants.com a qualified geotec commissioned ar	chnical e	enginee	r. Also,	boreho	le informa	ation sho	nding of all potential conditions present uld be read in conjunction with the geoto	and require inter echnical report fo	pretative assistan r which it was	ice from			Scale: Page:								



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method	Solid Stem Au	gers	Drilling Machine:	Buggy Mount	
Logged By:	SDP	Northing:	4821599	Date Started:	Mar 13/23
Reviewed By:	GW	Easting:	595754.3	Date Completed:	Mar 13/23
-					

	LITHOLOGY PROFILE	SOI	L SA	MPL	ING					LD T	-	-		LA	B TES	TING				OWN		s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	(m) H	ELEVATION (m)	\times	Other Pocke Field \	Strength Test It Penetro Vane (Inta Vane (Rer 80	meter ct)			Combus Total Or 100	ganic Vap	nic Vapo our (ppm 300 4	ur (%LEL)	Instrumentation Installation	0	8 GRAIN STRIE	k I SIZI	Ξ
Litholo	0.0 189.6	Sample	Sample	Recove	SPT "N	DEPTH (m)	ELEV	0		enetration	Testir DCPT 30	1g 40	PL	O Wate	r Content 20		LL 40	Instrun Installa	GR	SA	SI	CL
	0.2 TOPSOIL: 150 mm ^{189.4} FILL: Sand and silt, some clay, loose, brown, wet	SS	1	70	5	0	- 189	0 5		20					0 24					1		•
	Silty sand, very loose	ss	2	100	2	Ţ	Z	ر 2∖								9						
×××	1.5 188.1 SAND AND GRAVEL: Some silt, trace clay, compact, brown, wet	SS	3	45	10	1.5 —	-	10	þ					16 0								
	SANDY SILT GLACIAL TILL: Some clay trace gravel, cobbles and boulders,	SS	4	75	8	-	- 187.5	8¢						14 0					8	31	41	20
	loose, brownish grey, wet	SS	5	100	8	3	- ~	8 ¢						12 O								
						-	- 186															
	Grey 5.0 184.6	ss	6	15	4	4.5 —		64							23 〇							
1011 6-53	Borehole Terminated at 5.0 m																					
64	7 Welham Road, Unit 14				n compl	etion	of dr	illing: 0	.9 r	n			after au Elevat		moval: :	3.3 m.						
	arrie, Ontario L4N 0B7 T : (705) 719-7994 ww.geiconsultants.com	resente hnical e	ed do ne	ot cons r. Also,	titute a boreho	thorough le inform	ation sho	uld be re	f all p ead in	otential c conjunc	onditi tion w	ons preser ith the geo	t and req	uire inter	pretative	assistar	ice from			Scale Page		



Project Number:	2300805
Project Client:	Anatolia
Project Name:	Proposed Commercial Warehouse
Project Location:	6728 Sixth Line, Milton, ON
Drilling Location:	See Borehole Location Plan
Local Benchmark:	

Drilling Method:	Solid Stem Aug	ers	Drilling Machine:	Track Mount	
Logged By:	SDP	Northing:	4821712	Date Started:	May 1/23
Reviewed By:	GW	Easting:	595698.9	Date Completed: _	May 1/23

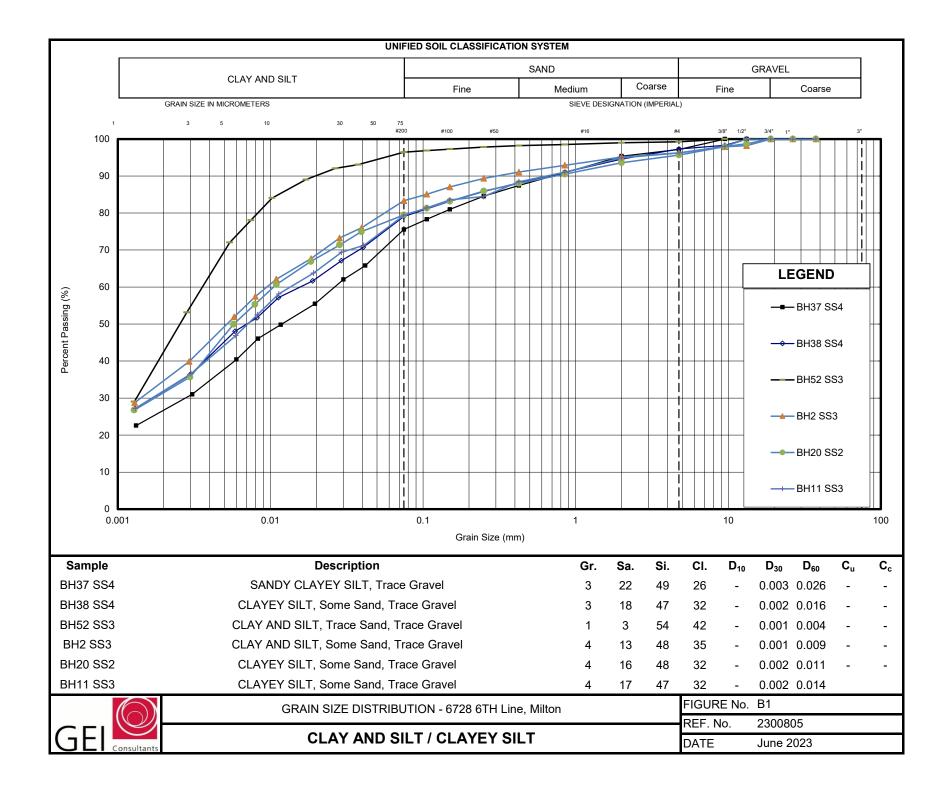
	LITHOLOGY PROFILE	SO	L SA	MPL	ING						STING			LAB	TES	TING					IENT	s
Lithology Plot	DESCRIPTION	Sample Type	Sample Number	Recovery (%)	SPT "N" Value	DEPTH (m)	ELEVATION (m)	× + ▲	Other Te Pocket P Field Var Field Var 40	st enetrome le (Intact) le (Remo 30 1) Ided) 20 1(Combusti Combusti Total Orga 00 2 Atte	ble Orgai	nic Vapou our (ppm) 00 4	ur (%LEL)) 00	Instrumentation Installation	0	8 GRAII STRII	& N SIZI BUTIC %)	E
Litholo	0.0 187.9	Sampl	Sampl	Recov	SPT "	DEPT	ELEV	0		etration Te	PT	0	PL -) Water	Content		— LL ю	Instrui Install	GR	SA	SI	CL
	^{0.2} TOPSOIL: 150 mm ^{187.8} FILL: Clayey silt, some sand, firm, brown, moist		1	65	4	0	187.5	0 4				0		0								
	^{1.0} - Silty sand seam and gravel, we ^{t86.9} -	SS	2	100	13	7	₽ ₽	1	39					19								
	SANDY SILT GLACIAL TILL: Some clay, trace gravel, inferred cobbles and	SS		100	10	1.5 -	-		100					11					First \	Nater	Strike	SS3
	boulders, compact to loose, brown, wet	55	3	100	16		- 186		160													
		SS	4	90	9		-	90	5					1 1 0								
			-			3-					<u> </u>			13 0								
		SS	5	85	17		- 184.5		17 Q	X				0								
							-			```												
1.00 (A)	Dense 5.0 182.9	SS	6	100	36	4.5 -	- 183				36 ⊘			2								
	Borehole Terminated at 5.0 m																					
	GEI CONSULTANTS 7 Welham Road, Unit 14						n compl	etion	of drilli	ng: 0.9	m	\sim		epth at Iwater		-	noval:	0.6 m.				
В	arrie, Ontario L4N 0B7 T : (705) 719-7994 Borehole details r	oresent	ed do n	ot cons	titute a	thorough	n understa	inding o	f all pote	ntial cor	ditions p	resent	and requ	ire interp	retative	assistan	ce from			Scale	:1:75	
w	ww.geiconsultants.com a qualified geotec commissioned an								ead in co	onjunctio	n with th	e geote	cnnical r	eport for	wnich it	was					1 of 1	

Geotechnical Investigation Proposed Commercial Warehouse Development 6728 Sixth Line, Milton, Ontario Project No. 2300805, June 14, 2023



Geotechnical Laboratory Testing



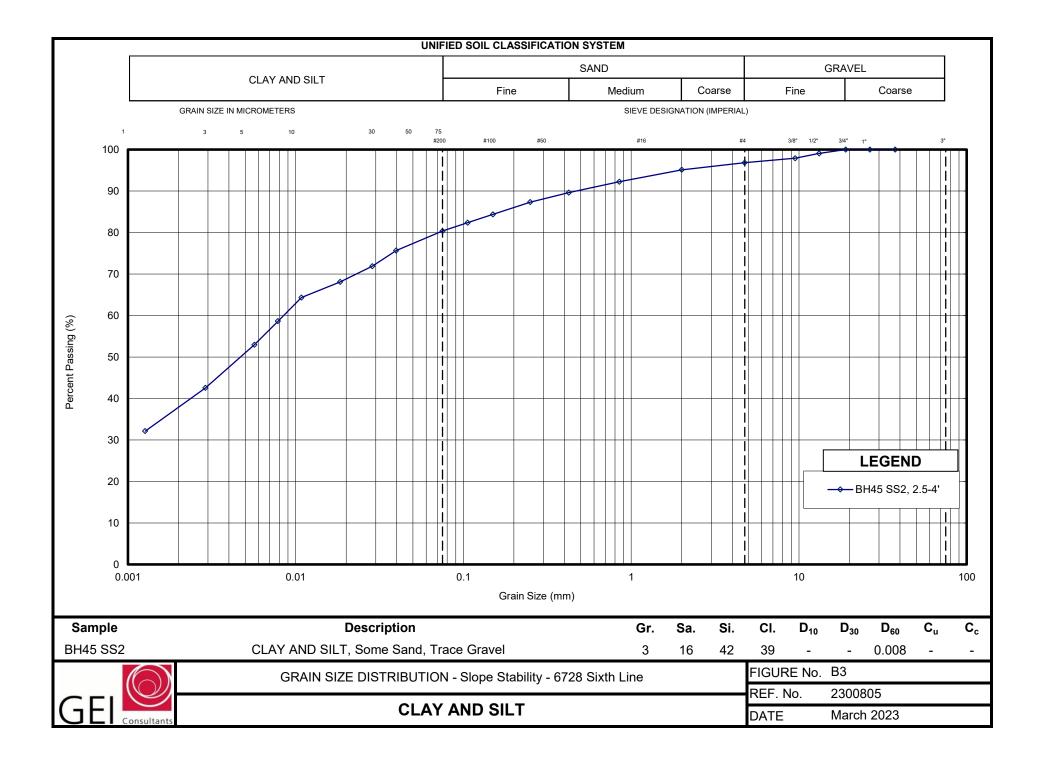


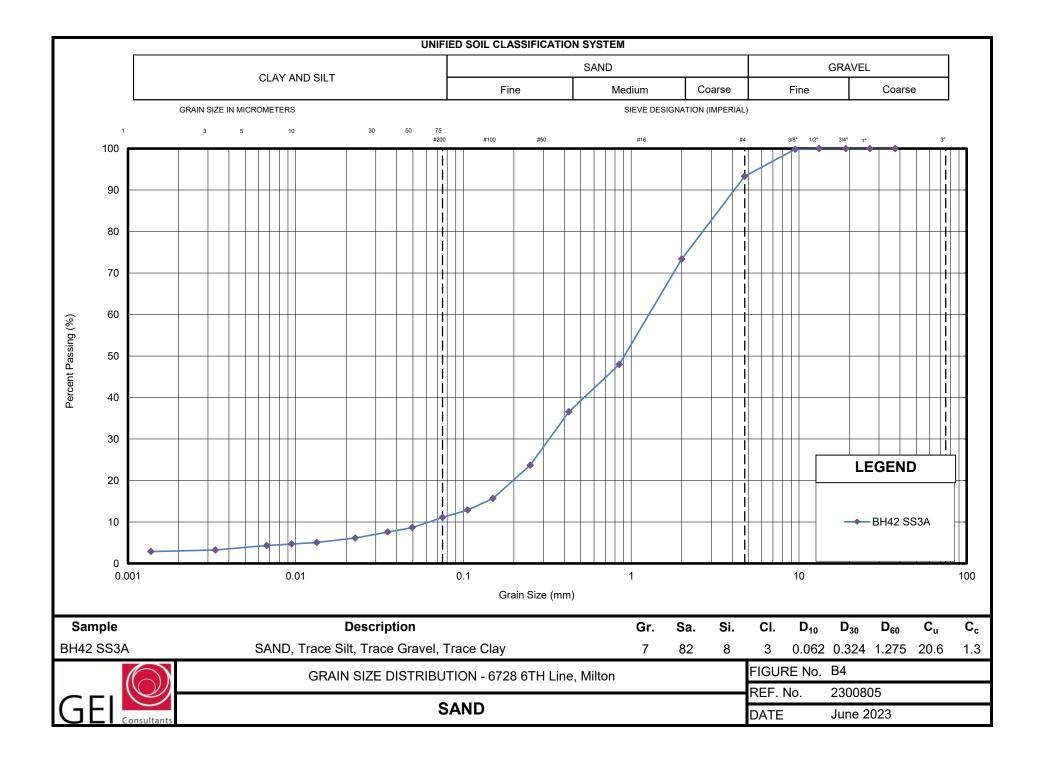


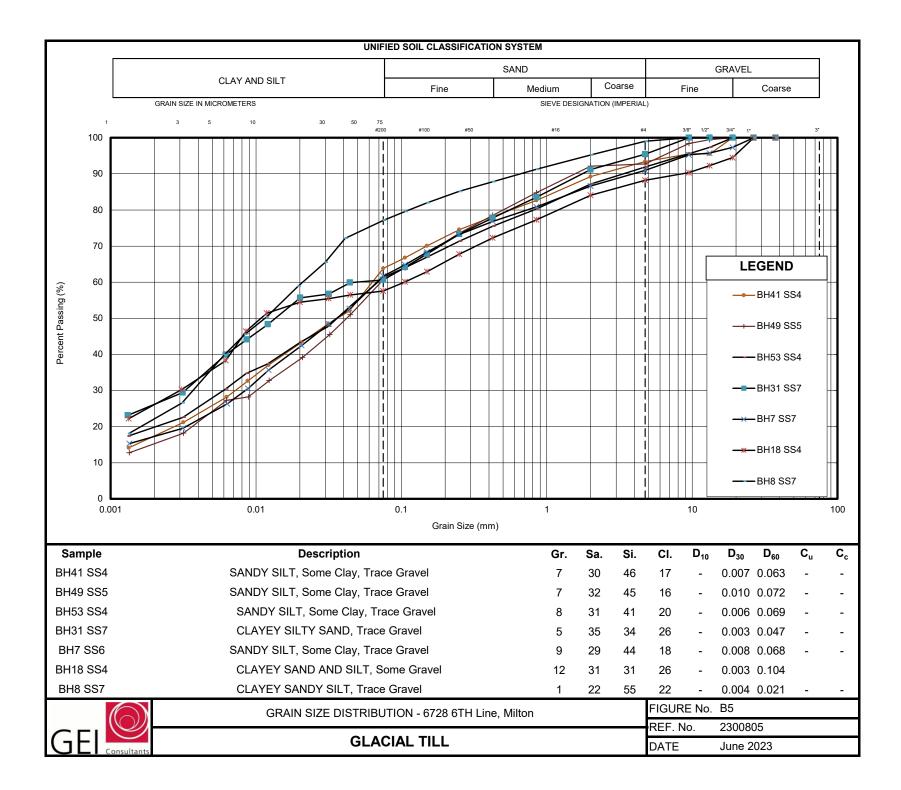
Atterberg Limits Report

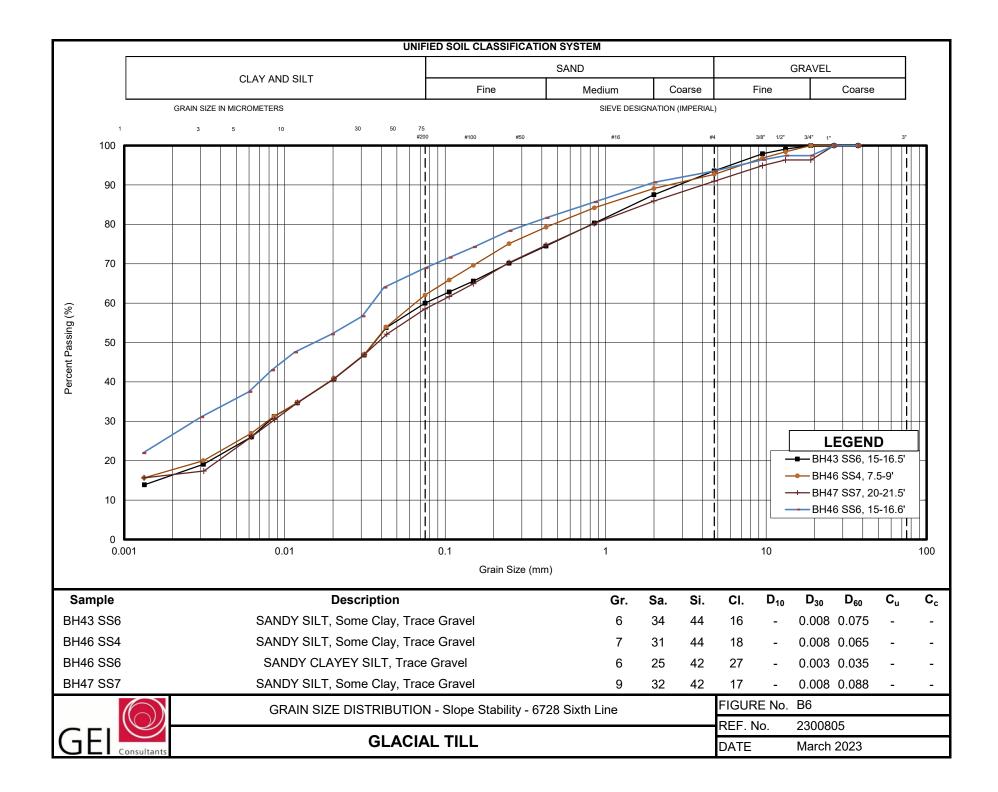
roject Na roject No lient:		Slope Stability - 6728 Sixth 2300805 Anatolia Investments Corpo	oration		Figure No.: Date Tested: Date Sampled:	B2 June 8, 2023 -		
				LE INFO				
SAMPL	E ID	BH38 SS4	LIQUID LIMIT (LL):	27.5	PLASTIC LIMIT (PL) 15.7	PLASTIC INDEX (PI)	11.8	X
SAMPL	e ID	BH52 SS3	LIQUID LIMIT (LL):	35.9	PLASTIC LIMIT (PL) 18.7	PLASTIC INDEX (PI)	17.2	*
SAMPL	e ID	BH11 SS3	LIQUID LIMIT (LL):	28.9	PLASTIC LIMIT (PL) 16.7	PLASTIC INDEX (PI)	12.2	+
			Plasti	icity Cl	hart			
	70							
	70							
	60							
	00					A-L	ine	
	- 0	CL				СН		
(II)	50							
dex								
ty In	40							
Plasticity Index (PI)								
Pla	30							
	20		Ж		1 + +	MH or C	ЭН	
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		M	ML or O					
	0	0 10 20	30 40	<u>ו</u>	50 60	70 80	90	100
		5 10 20			mit (LL)	, , , , , , , , , , , , , , , , , , , ,	50	100
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Report No.











Atterberg Limits Report

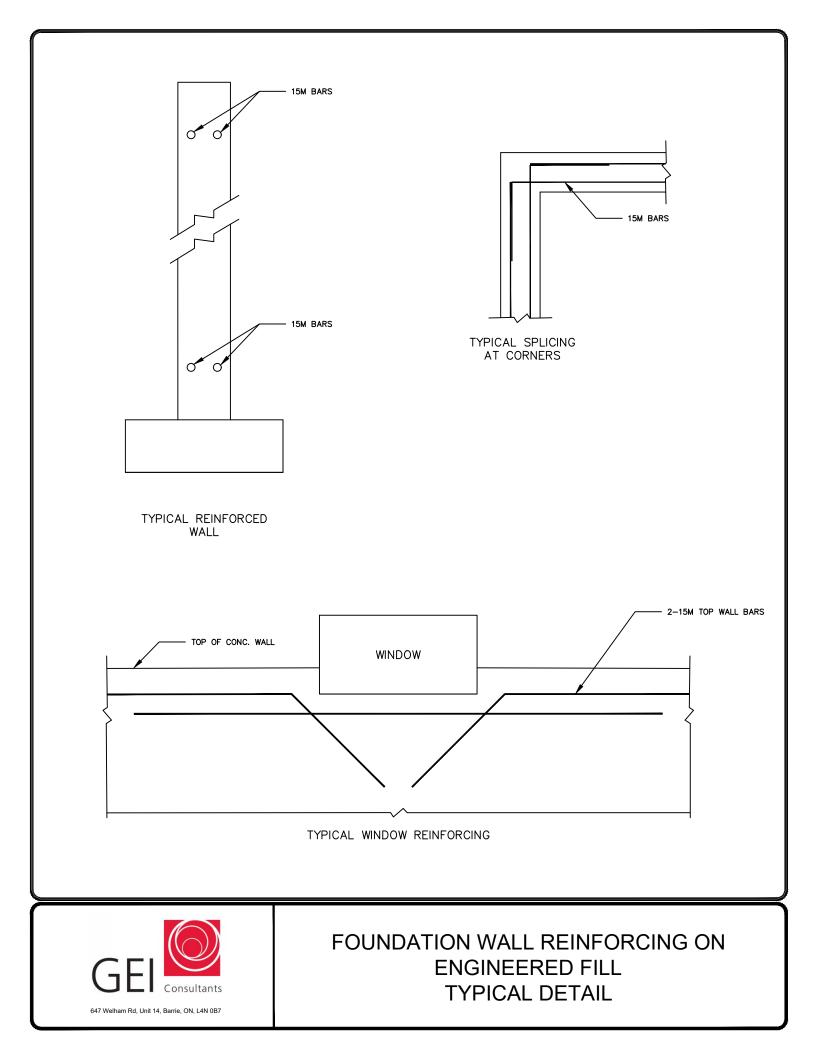
Project Name roject No.: Client:	: 6728 Sixth Line, Milton 2300805 Anatolia Investments Corpora			Figure N Date Tes Date Sar	sted:	B7 March 23, 2023 -		
		SAN	IPLE INFO	ORMATION				
SAMPLE ID	BH43 SS6, 15-16.6'	LIQUID LIMIT (LL):	20.4	PLASTIC LIMIT (PL)	13.8	PLASTIC INDEX (PI)	6.6	X
SAMPLE ID	BH45 SS2, 2.5-4'	LIQUID LIMIT (LL):	34	PLASTIC LIMIT (PL)	16.9	PLASTIC INDEX (PI)	17.1	
SAMPLE ID	BH46 SS4, 7.5-9'	LIQUID LIMIT (LL):	23.3	PLASTIC LIMIT (PL)	13.2	PLASTIC INDEX (PI)	10.1	0
SAMPLE ID	BH46 SS6, 15-16.6'	LIQUID LIMIT (LL):	23.6	PLASTIC LIMIT (PL)	8.0	PLASTIC INDEX (PI)	15.6	+
7	'0	Plas	sticity C	Chart				
6	0					A-Lii	ne	
(I I) x	.0 CL					СН		
Plasticity Index (PI)	0							
Plas 3	0							
2						MH or O	н	
1		ML or	OL -					
	0 10 20	30	40	50	60	70 80	90	100
			Liquid L	imit (LL).				
DISTRIBUT	ION:	Prepared By	: D. Gorr	у		Checked By: M. H-C	abal	

Geotechnical Investigation Proposed Commercial Warehouse Development 6728 Sixth Line, Milton, Ontario Project No. 2300805, June 14, 2023



Typical Details

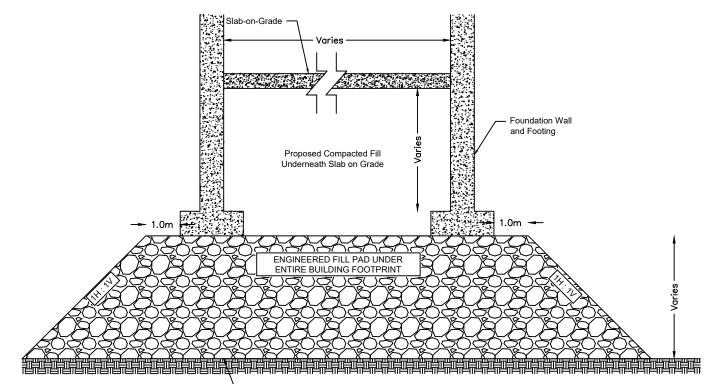




<u>Notes:</u>

- Engineered Fill compacted to 98% S.P.M.D.D and inspected under the full time supervision of CEE.
- 2. Interior non-structural compacted fill compacted to 98% S.P.M.D.D. with recommended part-time inspection.

S.P.M.D.D.- Standard Proctor Maximum Dry Density



 Exposed Competent Subgrade



ENGINEERED FILL PAD DIMENSIONING TYPICAL DETAIL

