



Tribal Development Management Services Inc.

**Preliminary Geotechnical Investigation
Proposed Mixed-Use Development (Phase 1
Land)
Britannia Road and First Line
Milton, Ontario**

Project Number
BRM-23015226-A0

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Executive Summary

The Proponent, Tribal Development Management Services Inc. (doing business as "Matthews"), is advancing the development of lands located adjacent to the CN Logistics Hub in the Town of Milton. Official Plan and Zoning By-law Amendments are being pursued to designate the subject lands as Industrial Area to be included within the Sustainable Halton Plan (SHP) Urban Area Boundary. The site will seek the entitlements associated with the Employment Zone, with site-specific provisions to support the intended development.

A Phase 1 Preliminary Geotechnical Investigation was completed for the subject lands to support the proposed industrial development. The investigation included drilling and sampling of boreholes across the site and laboratory testing to characterize subsurface soil and groundwater conditions. The subsurface profile generally consists of topsoil and reworked soils overlying very stiff to hard clayey glacial till, which extends to the depths investigated. Localized sandy silt till was encountered in limited areas. Groundwater levels were observed at variable depths across the site, and seasonal fluctuations are anticipated.

Based on the findings of the investigation, the subsurface conditions are considered suitable for the proposed industrial development, including slab-on-grade buildings, parking areas, and internal access roads. Preliminary recommendations are provided for site grading, foundation design, slab-on-grade construction, pavement structures, excavation practices, and seismic site classification. Shallow foundations bearing on native soils or engineered fill are considered feasible, subject to final grading and detailed design. The recommendations presented herein are preliminary and intended to support early-stage planning and entitlement activities.

1. Introduction

This report presents the results of a preliminary Geotechnical Investigation carried out at the site of a proposed Industrial development (Phase 1) in the Town of Milton, Ontario. The work was authorized by Mr. Davin McCully on behalf of Tribal Development Management Services Inc. (the Client).

The site is irregular in shape and is located on the northeast quadrant of Britannia Road and First Line. The Phase 1 property currently comprises agricultural and natural space and occupies an area of approximately 74.1 hectares (183 acres). Based on the Preliminary Parcel Identification Map dated 2023-06-20 provided by the Client, the proposed construction will comprise three (3) industrial slab on grade buildings without basement and associated access roads, loading and parking areas.

The purpose of this preliminary study was to determine the subsurface conditions at the site by drilling thirty (30) boreholes, designated 1-1 to 1-30, over the site. Based on the borehole information, preliminary geotechnical engineering guidelines for the design and construction of the proposed development are to be provided. Specifically, preliminary recommendations and/or comments regarding foundation type, design geotechnical reactions/resistances, groundwater conditions, excavation and backfill, slab-on-grade construction, permanent drainage requirements, earthquake considerations and pavement structures were to be provided.

Concurrent with this program, Hydrogeological Investigation and Environmental Site Assessment (ESA) have been carried out by EXP's hydrogeology and environmental teams. The results of hydrogeological investigation and ESA are presented under separate covers.

The comments and recommendations given in this report are based on the assumption that the above-described design concept will proceed into construction. If changes are made either in the design phase or during construction, this office must be retained to review these modifications. The result of this review may be a modification of our recommendations or the requirement of additional field or laboratory work to check whether the changes are acceptable from a geotechnical viewpoint.

2. Site Description

The site is irregular in shape and on the northeast quadrant of Britannia Road and First Line. The subject site is bordered by Britannia Road to the west, First Line to the south, Regional Road 25 to the north and existing farmland to the east, in the Town of Milton, Ontario. At the time of this field investigation, the site was covered by topsoil and vegetations.

3. Procedure

Drilling and sampling operations, carried out from February 20 to March 1 and June 7, 2024, were completed by a combination of auger and split-spoon techniques with drilling equipment owned and operated by a specialist contractor. All boreholes were advanced to depths of approximately 7.7 to 8.2 m below existing grades. A 50 mm diameter monitoring well was installed in each of Boreholes 1-5, 1-7 to 1-9, 1-18 to 1-22 and 1-23 to 1-27 permitting further monitoring of the groundwater levels as well as hydrogeological well testing and groundwater sampling. A 25 mm diameter piezometer was installed in each of Boreholes 1-4, 1-13, 1-16, 1-28 and 1-29 permitting further monitoring of the groundwater levels. Approximate borehole locations are shown on the attached Borehole Location Plan (Drawing No. 1).

A representative of EXP Services Inc. (EXP) was present throughout the fieldwork to monitor and direct the drill operations, and to record borehole information. Representative samples of the subsurface soils in the boreholes were recovered at regular intervals using conventional 50 mm O.D. split spoon sampling equipment driven in accordance with Standard Penetration Test procedures (ASTM D1586). All split spoon samples were returned to EXP's Brampton laboratory for testing which included moisture content and unit weight determinations on selected samples. Two (2) selected soil samples were subjected to grain size analyses and one (1) of these samples were subjected to Atterberg Limits testing. One (1) soil sample was also analyzed for pH and water soluble sulphate. The lab testing results are shown on the borehole logs as well as attached in Appendix A of this report. Water levels were monitored in the open boreholes prior to backfilling and subsequently on June 26 and July 4, 2024 in selected boreholes equipped with monitoring wells and piezometers.

The borehole locations were established in the field by EXP personnel. The tops of borehole elevations were surveyed by EXP using a Sokkia GCS3 Global Navigation Satellite System (GNSS) receiver based on information derived from the TopNET Live Network Service Global Positioning System (GPS).

4. Subsurface Conditions

The detailed soil profiles encountered in each borehole and the results of moisture content and unit weight determinations are indicated on the attached borehole logs. It should be noted that the soil boundaries indicated on the borehole logs are inferred from non-continuous sampling and observations during drilling. These boundaries are intended to reflect approximate transition zones for the purpose of geotechnical design and should not be interpreted as exact planes of geological change.

The "Notes on Sample Description" preceding the borehole logs form an integral part of and should be read in conjunction with this report.

The soil stratigraphy at the site consists of topsoil and reworked native soils over native deposits predominated by glacial till of clayey textures. Localized sandy silt till was also encountered. The following is a brief description of the soil conditions encountered in the boreholes during the investigation.

Topsoil

Topsoil with thicknesses ranging from 100 mm to 450 mm was encountered surficially in all boreholes. The thickness of the topsoil in each borehole is shown on the borehole log.

It should be noted that topsoil measurements were carried out at the borehole locations only and could differ at other locations on the site. If required, a more detailed test pit program should be carried out to more accurately quantify the amount of topsoil to be removed for construction purposes.

Reworked Till

Reworked till was encountered in all boreholes below the topsoil, except for Boreholes 1-7, 1-23 and 1-26 locations below the topsoil. The reworked till is considered to be disturbed parent material. The reworked till was fully penetrated at depth of 0.7 m. This material generally comprises brown silty clay with trace gravel and trace sand. Moisture contents in the reworked native soil varied between approximately 16 and 29 percent.

Clayey Silt Till to Silty Clay Till

Deposits of clayey silt till to silty clay till (clayey till) were encountered in all boreholes below the topsoil or reworked till. All boreholes were terminated in this stratum at approximately 7.7 to 8.2 m depth (~Elevation 172.9 to 178.1 m), except for Borehole 1-15. The clayey silt till deposit was fully penetrated at a depth of 5.5 m (~Elevation 177.7 m) in Borehole 1-15. The clayey till contains silty sand layers/seams, trace gravel, some sand and occasional cobbles and boulders. The clayey till is brown in colour with oxidized pockets, changing to grey with

depth. The consistency of the clayey till was assessed as stiff to hard, but generally very stiff to hard. Moisture content of the clayey till ranges from approximately 7 to 18 percent.

Grain size analyses on two (2) clayey till samples from Boreholes 1-9 and 1-25 indicated 3% to 5% gravel, 17% to 19% sand, 54% to 55% silt and 22% to 26% clay.

The Atterberg Limits testing carried out on the same sample from Borehole 1-9 yield 14% Plastic Limits (PL), 29% Liquid Limit (LL) and corresponding Plastic Index (PI) 15.

Sand Silt Till

Localized deposit of sandy silt till was encountered in Borehole 1-15. Borehole 1-15 was terminated at a depth of approximately 7.7 m (~Elevation 175.5 m) in this deposit. The sandy silt till is brown in colour. This soil exists in a very dense condition. Moisture content of the sandy silt till was recorded at approximately 9 to 11 percent.

Groundwater Conditions

Groundwater conditions were assessed in the open boreholes during the course of the fieldwork and subsequent stabilized groundwater readings were taken in monitoring wells in selected boreholes on June 26 and July 4, 2024. Short term and subsequent groundwater levels are recorded on the attached borehole logs.

Upon completion of drilling, free water was observed in Boreholes 1-6, 1-7, 1-10, 1-12 and 1-13 at depths of approximately 2.4 to 7.3 m below existing grade. The remaining boreholes were dry upon completion of augering. The subsequent groundwater levels were recorded between about 0.2 to 3.3 m below existing grade in monitoring wells/piezometers installed at the site. A summary of groundwater observations in the monitoring wells is presented in the following Table 1.

Table 1: Groundwater Level Readings

Borehole No.	Date of Measurement	Water Level (m)	Water Elevation (m)	Note
1-4	June 26, 2024 July 4, 2024	0.23 0.25	181.4 181.4	25 mm Piezometer
1-5	June 26, 2024 July 4, 2024	0.37 0.35	183.5 183.5	50 mm Monitoring Well
1-7	June 26, 2024 July 4, 2024	0.29 0.36	183.0 182.9	50 mm Monitoring Well
1-8	June 26, 2024 July 4, 2024	0.49 0.54	184.1 184.0	50 mm Monitoring Well
1-9	June 26, 2024 July 4, 2024	1.13 1.71	181.4 180.8	50 mm Monitoring Well
1-13	June 26, 2024 July 4, 2024	2.13 1.76	180.3 180.6	25 mm Piezometer
1-16	June 26, 2024 July 4, 2024	3.28 0.84	180.1 182.5	25 mm Piezometer
1-18	June 26, 2024 July 4, 2024	1.87 1.91	181.7 181.6	50 mm Monitoring Well
1-19	June 26, 2024 July 4, 2024	0.38 0.36	182.9 182.9	50 mm Monitoring Well
1-20	June 26, 2024 July 4, 2024	0.25 1.63	182.9 181.5	50 mm Monitoring Well
1-21	June 26, 2024 July 4, 2024	0.17 0.21	184.5 184.5	50 mm Monitoring Well
1-22	June 26, 2024 July 4, 2024	0.49 2.21	183.7 182.0	50 mm Monitoring Well
1-23	June 26, 2024 July 4, 2024	0.42 0.43	185.9 185.9	50 mm Monitoring Well
1-24	June 26, 2024 July 4, 2024	0.51 1.09	183.9 183.3	50 mm Monitoring Well
1-25	June 26, 2024 July 4, 2024	0.24 0.62	185.0 184.6	50 mm Monitoring Well
1-26	June 26, 2024 July 4, 2024	0.40 3.15	185.6 182.9	50 mm Monitoring Well
1-27	June 26, 2024 July 4, 2024	0.80 1.85	183.3 182.3	50 mm Monitoring Well
1-28	June 26, 2024 July 4, 2024	0.26 0.24	185.3 185.3	25 mm Piezometer
1-29	June 26, 2024 July 4, 2024	0.90 0.88	183.5 183.5	25 mm Piezometer

The groundwater elevations reflect the conditions at the time of the investigation. Seasonal fluctuation of the groundwater levels at the site should be anticipated.

For detailed groundwater information, it should be referred to the hydrogeological investigation report.

5. Preliminary Geotechnical Assessment

5.1 Site Grading

The site grading plan is unknown at the time of preparing this preliminary geotechnical investigation report. Given the range of collar elevations of the boreholes drilled within the proposed development limits of approximately 181.1 to 186.3 m (relief of approximately 5.2 m), it is anticipated that some regrading (cut and fill operations) will be carried out at the site. The following procedures are recommended for the construction of fill sections for pavement and building areas at the site, where required.

- All existing topsoil, reworked till, vegetation and associated roots, and obviously disturbed native materials should be removed from beneath the proposed building and pavement areas.
- The exposed subgrade surface should be proofrolled with a heavy vibratory roller and examined by a geotechnical engineer. Any soft areas detected during the proofrolling process should be subexcavated.
- The area can then be brought up to the final subgrade level with approved on-site or imported material placed in lifts not exceeding 200 mm and compacted to the following requirements:
 - I. Within the building areas - 100 percent for foundation support and a minimum of 98 percent standard Proctor maximum dry density (SPMDD) for slab-on-grade support.
 - II. Pavement areas - 98 percent SPMDD for the upper 600 mm and a minimum of 95 percent SPMDD to within 600 mm of final subgrade level.
 - III. General backfill including trench backfill and backfill adjacent to foundation walls - minimum of 98 percent SPMDD.
- All backfilling and compaction operations should be monitored on a full-time basis by qualified geotechnical personnel to approve material, evaluate placement operations and confirm the specified degree of compaction is achieved uniformly throughout the fill.
- Where free-draining backfill is required, or in confined areas, imported granular material conforming to OPSS Granular 'B' is recommended.

5.2 Foundation Considerations

Soil conditions on the site are considered suitable for construction of the proposed industrial buildings and associated loading and parking areas. Since final site grades have not yet been established, two (2) foundation schemes are being presented for consideration, namely footings on native soils and footings on engineered fill.

5.2.1 Footings on Native Soil

Based on the results of the investigation, conditions are suitable to support the proposed structure at Boreholes 1-5 to 1-9, 1-18 to 1-27, which were drilled within/near the proposed building footprint. At these locations the proposed structure can be supported on conventional spread and strip footings or drilled piers founded at a depth of 1.2 to 1.5 m below the existing grade (~Elevation 208.1 to 210.9 m) on the very stiff to hard clayey tills. The footings/piers can be designed for a geotechnical reaction at Serviceability Limit States (SLS) of 300 kPa and factored geotechnical resistance of 450 kPa at Ultimate Limit States (ULS), subject to geotechnical inspection during construction.

It should be noted that a weaker layer of silty clay till stratum was encountered in Boreholes 1-18 to 1-24. Depending on the final site grading plan and foundation design, It is recommended that the footings in this area be founded as high as possible to avoid overstressing the lower weaker soils. Once the site grading plan and foundation design is available, EXP should be retained to review the site grading plan and foundation design. A more detailed investigation will be carried out and further recommendations will be provided as appropriate.

The following Table 2 shows the highest elevations at the borehole locations where the afore mentioned geotechnical reactions/resistances can be applied.

**Table 2: Highest Elevation at Borehole Locations Where
Recommended Geotechnical Reaction/Resistance can be Applied**

Borehole No.	Existing Grade Elevation (m)	Spread and Strip Footing/Drilled Pier SLS 300 kPa/ULS 450 kPa ~ Elevation (Depth Below Existing Grade (m))
1-5	183.8	182.3 (1.5)
1-6	183.3	182.1 (1.2)
1-7	183.3	181.8 (1.5)
1-8	184.6	183.4 (1.2)
1-9	182.5	181.3 (1.2)
1-18	184.6	183.4 (1.2)
1-19	183.3	182.1 (1.2)
1-20	183.1	181.6 (1.5)
1-21	184.7	183.5 (1.2)
1-22	184.2	182.7 (1.5)
1-23	186.3	185.1 (1.2)
1-24	184.4	183.2 (1.2)
1-25	185.2	184.0 (1.2)
1-26	186.0	184.8 (1.2)
1-27	184.1	182.9 (1.2)

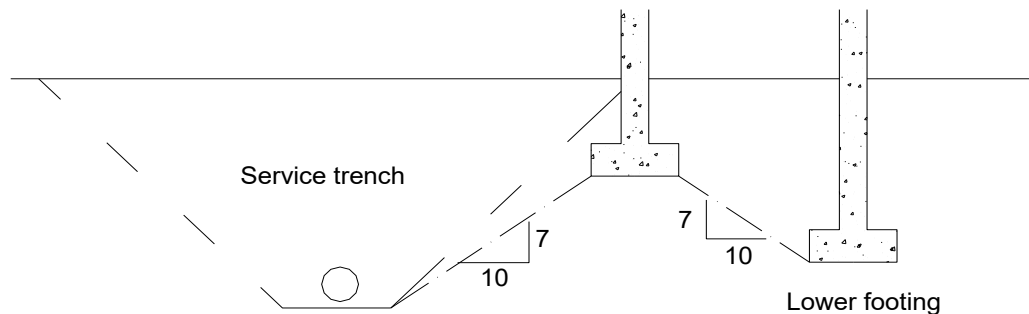
5.2.2 Footings on Engineered Fill

Depending on the final grading plan of the site, the proposed industrial building structure may also be supported on footings founded on engineered fill developed over the competent native soils. A Geotechnical Reaction of 150 kPa at SLS and factored Geotechnical Resistance of 225 kPa at ULS can be utilized for footings placed on engineered fills. It is recommended the engineered fill be constructed to design floor subgrade level prior to installation of footings. This will ensure the engineered fill placed will be suitable for foundations as well as slab-on-grade support. Engineered fill construction in the winter months is not recommended.

Recommendations for construction of Engineered Fill are summarized in Appendix B.

5.3 Foundations General

Footings/drilled piers at different elevations should be located such that higher footings/drilled piers are set below a line drawn up at 10 Horizontal to 7 Vertical from the near edge of the lower footing/drilled piers. This concept should also be applied to excavations for new foundations in relation to existing foundations or underground services. This concept is illustrated in the following sketch.



FOOTINGS NEAR SERVICE TRENCHES OR AT DIFFERENT ELEVATIONS

All footings/drilled piers exposed to freezing conditions must be provided with a minimum of 1.2 m of earth cover or equivalent insulation for frost protection, depending on the final grade requirements.

Provided that the soil is not disturbed due to groundwater, precipitation, traffic, etc., and the aforementioned geotechnical reactions/resistances are not exceeded, then total and differential settlements should be small and within the normally tolerated limits of 25 mm and 19 mm, respectively.

As mentioned previously, all footing bases must be inspected prior to pouring concrete. If the footings are not poured on the same day of the excavation work, the excavated footing bases should be covered with a 50 mm thick mud slab immediately after inspection and cleaning, in order to avoid disturbance of the founding soil due to construction activity and weathering.

The recommended bearing capacities have been calculated by EXP from the borehole information for the design stage only. The investigation and comments are necessarily on-going as new information of underground conditions becomes available. For example, it should be appreciated that modifications to bearing levels may be required if unforeseen subsoil conditions are revealed after the excavation is exposed to full view or if final design

decisions differ from those assumed in this report. For this reason, this office should be retained to review final foundation drawings and to provide field inspections during the construction stage. As stated before, the results of this geotechnical study should therefore be regarded as very preliminary at best due to the limited number of boreholes drilled to the required depth for the proposed development.

5.4 Floor Slabs and Permanent Drainage

The floor slabs can be constructed as a slab-on-grade on the native clayey till or on an engineered fill subgrade. A modulus of subgrade reaction k of 50 and 30 MPa/m (~180 and 110 psi/in) on native soils and engineered fill respectively, may be used for the design of the floor slabs.

All topsoil, reworked till, vegetation and associated roots, unsuitable material and other deleterious material should be removed from the entire underfloor areas. Following site grading, the exposed subgrade surface should then be thoroughly proof-rolled with a heavy vibratory roller. Any soft spots detected should be dug out and replaced with compactible fill in the manner described in Section 5.6 - "Backfill Considerations" section of this report. The site can then be filled to the required grades as outlined in Section 5.1 – "Site Grading".

A 200 mm layer of 19 mm clear crushed stone should be placed between the prepared subgrade and the floor slab to serve as a moisture barrier. Also, within any unheated areas and entrances to the building, Styrofoam insulation should be provided below the floor slab and against the foundation walls to protect against frost heave.

Based on the soil and groundwater conditions at the site, underfloor drains will not be required. Perimeter drains are not required if the floor slab of building is set at least 200 mm above the existing exterior grade. Around the perimeter of the building, the ground surface should be sloped on a positive grade away from the structure to promote surface water run-off and reduce groundwater infiltration adjacent to the foundations.

5.5 Excavations and Groundwater Control

All excavation must be carried out in accordance with the most recent Occupational Health and Safety Act. The glacial till is classified as Type 2 soil. Where loose/soft materials are encountered locally, or within zones of persistent seepage at depth, it may be necessary to flatten the side slopes.

It should be noted that the presence of cobbles and boulders in glacial till deposits may influence the progress of excavation. Consequently, provisions should be made in the contract documents to cover any delays caused by these obstructions.

The final excavation depth is unknown, it is anticipated that all excavation would be carried out within clayey till. No major problems with groundwater are anticipated. Some seepage of free water from more pervious seams and layers within the tills should be anticipated during construction. It should be possible to control and remove any such seepage by pumping from temporary sumps and ditches and/or oversized excavations.

5.6 Backfill Considerations

Backfill used to satisfy underfloor slab requirements, in footings and service trenches, etc., should be compactible fill, i.e., inorganic soil with its moisture content close to its optimum moisture content determined in the standard Proctor maximum dry density test. For ease of compaction and quality control in confined areas, sand fill such as Ontario Provincial Standard Specifications (OPSS) 1010 Granular 'B' is recommended. The backfill should be placed in lifts not more than 200 mm thick in the loose state, each lift being compacted to at least 98 percent SPMD under the floor slab and 95 percent elsewhere, before subsequent lifts are placed. The degree of compaction achieved in the field should be checked by in-place density tests.

The majority of excavated material will likely consist of the glacial till. In general, the excavated material can be reused for structural backfill subject to the removal of any topsoil inclusions or other obviously unsuitable material. Some water content adjustment may be required for efficient compaction depending upon the material type and weather conditions at the time of construction. Any organic or excessively wet or otherwise deleterious material should not be used for backfill purposes. Any shortfall of suitable on-site excavated material can be made up with imported granular material, OPSS Granular 'B' or equivalent.

In general, the on-site soils are not free draining and therefore should not be used where this characteristic is required, or in confined areas. Imported granular material conforming to OPSS Granular 'B' would also be suitable for these purposes.

5.7 Earthquake Considerations

The recommendations for the geotechnical aspects to determine the earthquake loading for design in accordance with Section 4.1.8 in the Ontario Building Code (OBC) 2024, are presented below.

5.7.1 Subsoil Conditions

The subsoil and groundwater information at this site have been examined in relation to Section 4.1.8.4 of the OBC (2024). The subsoils generally consist of glacial tills interbedded with sandy/silty layers seams. It is anticipated that the floor slabs of the proposed structures will be founded on clayey tills or engineered fill.

5.7.2 Depth of Boreholes

Table 4.1.8.4.A. Site Classification for Seismic Site Response in OBC 2024 indicated that to determine the site classification, the average properties in the top 30 m (below the lowest basement level) are to be used. Site Classification can be determined using the average shear wave velocity (V_{a30}) as per the classifications stated in Table 4.1.8.4-B. If in-situ measurements are not available, the site designation shall be determined based on the energy-corrected average standard penetration resistance (SPT) N_{60} or the average undrained shear strength S_u in accordance with Table 4.1.8.4.B.

There are no shear wave measurements carried out at this site and therefore, the Site Class will be determined based on the energy-corrected average SPT. The boreholes advanced at this site terminated at depths of about 7.7 to 8.2 m below existing grade. Therefore, the site classification recommendation would be based on the available information as well as our interpretation of conditions below the boreholes based on our knowledge of the soil conditions in the area.

5.7.3 Site Classification

Based on the above assumptions and currently available information, the Site Class for the proposed building is “D” as per Table 4.1.8.4.B, Site Classification for Seismic Site Response, OBC 2024. Shear wave velocity measurements can be carried out to determine if a more favorable Site Classification can be used.

5.8 Parking Areas and Internal Access Roads

The recommended pavement structures provided in Table 3 are based upon an estimate of the subgrade soil properties determined from visual examination and textural classification of the soil samples. A functional design life of 8 to 10 years has been used to establish the pavement recommendations. This represents the number of years to the first rehabilitation, assuming regular maintenance is carried out. Other thickness combinations can be used provided the Granular Base Equivalency (GBE) is maintained and any minimum component thickness specified by the Town of Milton is met.

Table 3: Recommended Pavement Structure Thickness

Pavement Layer	Compaction Requirements	Light-Duty Parking (Cars)	Heavy Duty Traffic (Trucks) Loading Docks and Access Road
Asphaltic Concrete (OPSS 310)	Minimum 92 % MRD**	40 mm HL3 50 mm HL8	40 mm HL3 85 mm HL8
19 mm Crusher Run Limestone	100 % SPMDD*	150 mm	150 mm
50 mm Crusher Run Limestone	100 % SPMDD*	300 mm	450 mm

* Denotes standard Proctor maximum dry density, ASTM-D698

** Denotes Maximum Relative density, MTO LS-264

Where rigid pavements are required such as the truck loading dock apron area and dolly pads for trailer parking, recommended pavement structures are summarized in Table 4.

Table 4: Recommended Pavement Structure Thickness (Rigid)

Pavement Layer	Compaction Requirements	Concrete Pads Adjacent to Loading Docks	Concrete Dolly Pads
Concrete CSA Class C-2 32 MPa	-	230 mm	200 mm (4)
19 mm Crusher-run Limestone (OPSS 1010)	100% SPMDD (2)	200 mm	70 mm
50 mm Crusher-run Limestone (OPSS 1010)	100% SPMDD (2)	-	500 mm

Notes: (2) Denotes standard Proctor maximum dry density, MTO LS-706 (Procedure 3)
(4) The concrete dolly pads should be 200mm thick with shrinkage steel.

The concrete for this project should be C-2 Class as a minimum, i.e., 32 MPa, 0.45 maximum water/cement ratio. The nominal aggregate size in the mix should be 37.5 mm and the air content should range from 4 to 7%.

The subgrade must be compacted to 98 percent SPMDD for at least the upper 600 mm.

The long-term performance of the pavement structure is highly dependent upon the subgrade support conditions. Stringent construction control procedures should be maintained to ensure uniform subgrade moisture and density conditions are achieved. In addition, the need for adequate drainage cannot be over-emphasized. The finished pavement surface should be free of depressions and should be sloped (preferably at a minimum grade of two percent) to provide effective surface drainage toward catch basins. Subdrains should be installed to intercept excess subsurface moisture and to prevent subgrade softening. This is particularly important in heavy-duty pavement areas.

Additional comments on the construction of parking areas and access roadways are as follows:

1. As part of the subgrade preparation, proposed parking areas and access roadways should be stripped of topsoil, reworked tills and associated rootlets and other obvious objectionable material. Fill required to raise the grade to design elevations should conform to backfill requirements outlined in previous sections of this report. The subgrade should be proof-rolled in the full-time presence of qualified geotechnical personnel. Soft or spongy subgrade areas should be sub-excavated and properly replaced with suitable approved backfill compacted to 98 percent SPMDD. The final subgrade surface should then be properly shaped and crowned.
2. Assuming that satisfactory cross-falls have been provided for subdrainage, subdrains extending from and between catch basins may be satisfactory. Further, subdrains should also be installed along the perimeter of pavement areas as well as around landscaped areas.
3. The most severe loading conditions on light-duty pavement areas and the subgrade may occur during construction. Consequently, special provisions such as restricted access lanes, half-loads during paving, etc., may be required, especially if construction is carried out during unfavorable weather.
4. To minimize the problems of differential movement between the pavement and catchbasins/manholes due to frost action, the backfill around the structures should consist of free-draining granular.

5. To prevent water ponding at the lower pavement areas of the loading docks, it is recommended that catchbasins be provided to drain the surface run-offs.

5.9 Potential for Sulphate Attack

One (1) native soil sample identified as Borehole 1-16 SS3 was analyzed for pH and water soluble sulphate. The pH value of the soil sample is 7.92. The soluble sulphate content in the samples analyzed was reported as 960 µg/g (0.096%). Table 3 of the Canadian Standards Association (CSA) A.23.1-09 lists 0.1 % sulphate as the minimum concentration of sulphate in soil that warrants additional requirements for concrete. At the measured concentration, the degree of exposure to sulphate attack is considered to be “negligible” and therefore normal Portland cement (Type 10) can be used in subsurface concrete. The Certificate of Analysis was attached in Appendix C of this report.

6. General Comments

The information presented in this report is based on a limited investigation designed to provide information to support an overall assessment of the current geotechnical conditions of the subject property. The conclusions presented in this report reflect site conditions existing at the time of the investigation.

EXP should be retained for a general review of the final design and specifications to verify that this report has been properly interpreted and implemented. If not accorded the privilege of making this review, EXP will assume no responsibility for interpretation of the recommendations in the report.

The comments given in this report are preliminary in nature and intended only for the guidance of design engineers. More boreholes are required to determine the localized underground conditions between the widely spaced boreholes which may affect construction costs, techniques, sequencing, equipment, scheduling, etc. Contractors bidding on or undertaking the works using this preliminary information should, in this light, decide on their own investigations as well as their own interpretations of the factual borehole results so that they may draw their own conclusions as to how the subsurface conditions may affect them.

More specific information, with respect to the conditions between samples, or the lateral and vertical extent of materials, may become apparent during excavation operations. Consequently, during the future development of the property, conditions not observed during this investigation may become apparent; should this occur, EXP should be contacted to assess the situation, and additional testing and reporting may be required. EXP has qualified personnel to provide assistance in regard to future geotechnical and environmental issues related to this property.

We trust this report is satisfactory for your purposes. Should you have any questions, please do not hesitate to contact this office.

EXP Services Inc.



Jack Li, P. Eng.
Senior Engineer
Geotechnical Division



Stephen S. M. Cheng, P.Eng.
Discipline Manager
Geotechnical Division

Drawings:
Borehole Location Plan
Notes on Sample Descriptions
Borehole Logs

Notes on Sample Descriptions and Soil Types

Drawing 1A

1. All sample descriptions included in this report follow the Canadian Foundations Engineering Manual soil classification system. This system follows the standard proposed by the International Society for Soil Mechanics and Foundation Engineering. Laboratory grain size analyses provided by **exp** also follow the same system. Others may use different classification systems; one such system is the Unified Soil Classification. Please note that, with the exception of those samples where a grain size analysis has been made, all samples are classified visually. Visual classification is not sufficiently accurate to provide exact grain sizing or precise differentiation between size classification systems.

ISSMFE SOIL CLASSIFICATION													
CLAY	SILT			SAND			GRAVEL			COBBLES	BOULDERS		
	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE	FINE	MEDIUM	COARSE				
	0.002	0.006	0.02	0.06	0.2	0.6	2.0	6.0	20	60	200		
EQUIVALENT GRAIN DIAMETER IN MILLIMETERS													
CLAY (PLASTIC) TO SILT (NONPLASTIC)				FINE		MEDIUM		COARSE		FINE		COARSE	
				SAND						GRAVEL			
UNIFIED SOIL CLASSIFICATION													

UNIFIED SOIL CLASSIFICATION

2. **Fill:** Where fill is designated on the borehole log it is defined as indicated by the sample recovered during the boring process. The reader is cautioned that fills are heterogeneous in nature and variable in density or degree of compaction. The borehole description may therefore not be applicable as a general description of site fill materials. All fills should be expected to contain obstruction such as wood, large concrete pieces or subsurface basements, floors, tanks, etc., none of these may have been encountered in the boreholes. Since boreholes cannot accurately define the contents of the fill, test pits are recommended to provide supplementary information. Despite the use of test pits, the heterogeneous nature of fill will leave some ambiguity as to the exact composition of the fill. Most fills contain pockets, seams, or layers of organically contaminated soil. This organic material can result in the generation of methane gas and/or significant ongoing and future settlements. Fill at this site may have been monitored for the presence of methane gas and, if so, the results are given on the borehole logs. The monitoring process does not indicate the volume of gas that can be potentially generated nor does it pinpoint the source of the gas. These readings are to advise of the presence of gas only, and a detailed study is recommended for sites where any explosive gas/methane is detected. Some fill material may be contaminated by toxic/hazardous waste that renders it unacceptable for deposition in any but designated land fill sites; unless specifically stated the fill on this site has not been tested for contaminants that may be considered toxic or hazardous. This testing and a potential hazard study can be undertaken if requested. In most residential/commercial areas undergoing reconstruction, buried oil tanks are common and are generally not detected in a conventional geotechnical site investigation.
3. **Till:** The term till on the borehole logs indicates that the material originates from a geological process associated with glaciation. Because of this geological process the till must be considered heterogeneous in composition and as such may contain pockets and/or seams of material such as sand, gravel, silt or clay. Till often contains cobbles (60 to 200 mm) or boulders (over 200 mm). Contractors may therefore encounter cobbles and boulders during excavation, even if they are not indicated by the borings. It should be appreciated that normal sampling equipment cannot differentiate the size or type of any obstruction. Because of the horizontal and vertical variability of

till, the sample description may be applicable to a very limited zone; caution is therefore essential when dealing with sensitive excavations or dewatering programs in till materials.

4. Excerpt from "OHSA Regulations for Construction Projects," Part III, Section 226:

- **Soil Types**

Type 1 Soil

- a) is hard, very dense and only able to be penetrated with difficulty by a small sharp object;
- b) has a low natural moisture content and a high degree of internal strength;
- c) has no signs of water seepage; and
- d) can be excavated only by mechanical equipment.

Type 2 Soil

- a) is very stiff, dense and can be penetrated with moderate difficulty by a small sharp object;
- b) has a low to medium natural moisture content and a medium degree of internal strength; and
- c) has a damp appearance after it is excavated.

Type 3 Soil

- a) is stiff to firm and compact to loose in consistency or is previously excavated soil;
- b) exhibits signs of surface cracking;
- c) exhibits signs of water seepage;
- d) if it is dry, may run easily into a well-defined conical pile; and
- e) has a low degree of internal strength.

Type 4 Soil

- a) is soft to very soft and very loose in consistency, very sensitive and upon disturbance is significantly reduced in natural strength;
- b) runs easily or flows, unless it is completely supported before excavating procedures;
- c) has almost no internal strength;
- d) is wet or muddy; and
- e) exerts substantial fluid pressure on its supporting system. O. Reg. 213/91, s. 226.

Log of Borehole 1- 1

Project No. BRM-23015226-A0

Drawing No. 2

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 20,2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

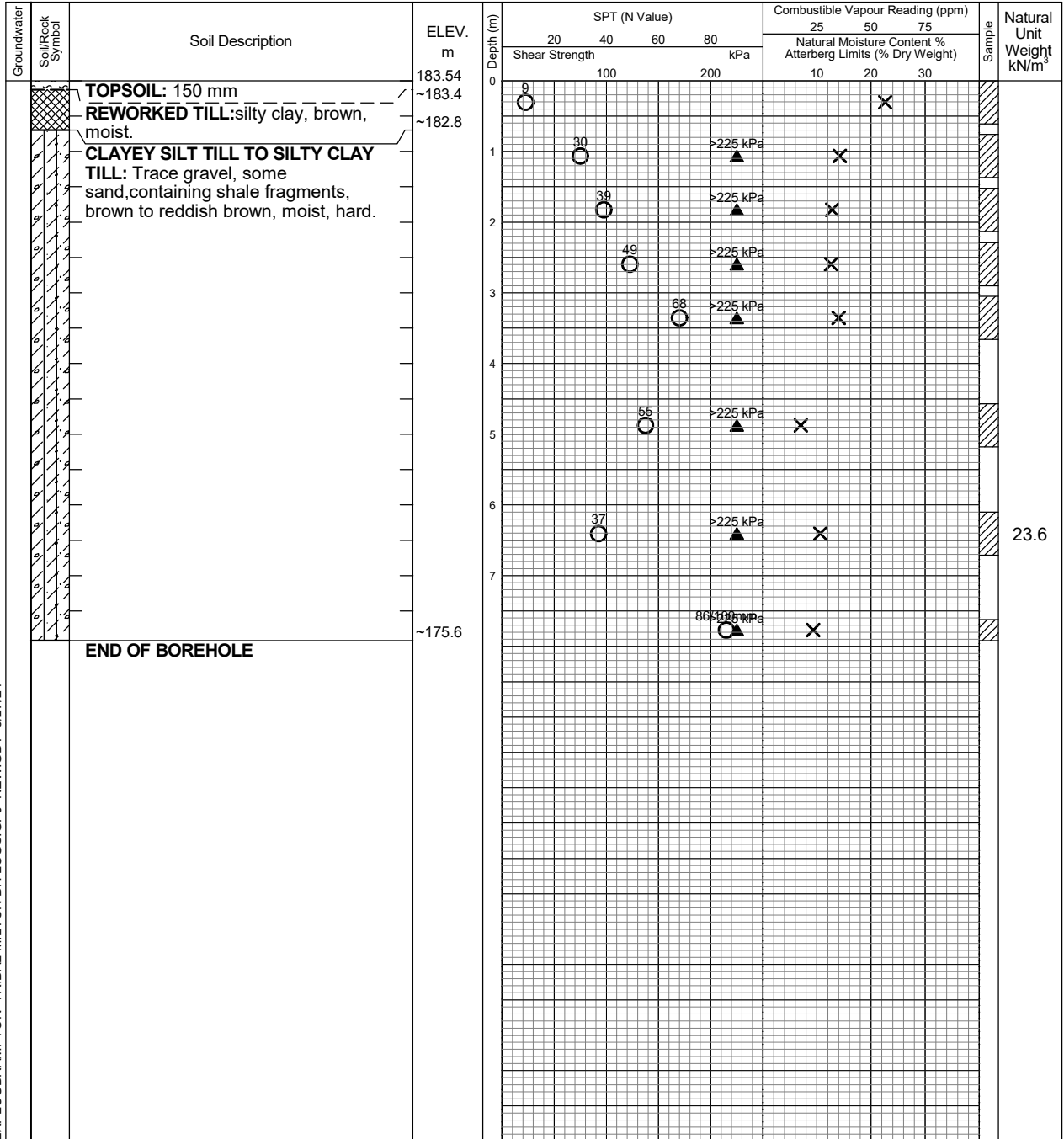
Undrained Triaxial at

% Strain at Failure

Penetrometer

Drill Type: Solid Auger 110mm

Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	Dry	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	Dry	Open

Log of Borehole 1- 3

Project No. BRM-23015226-A0

Drawing No. 4

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 23,2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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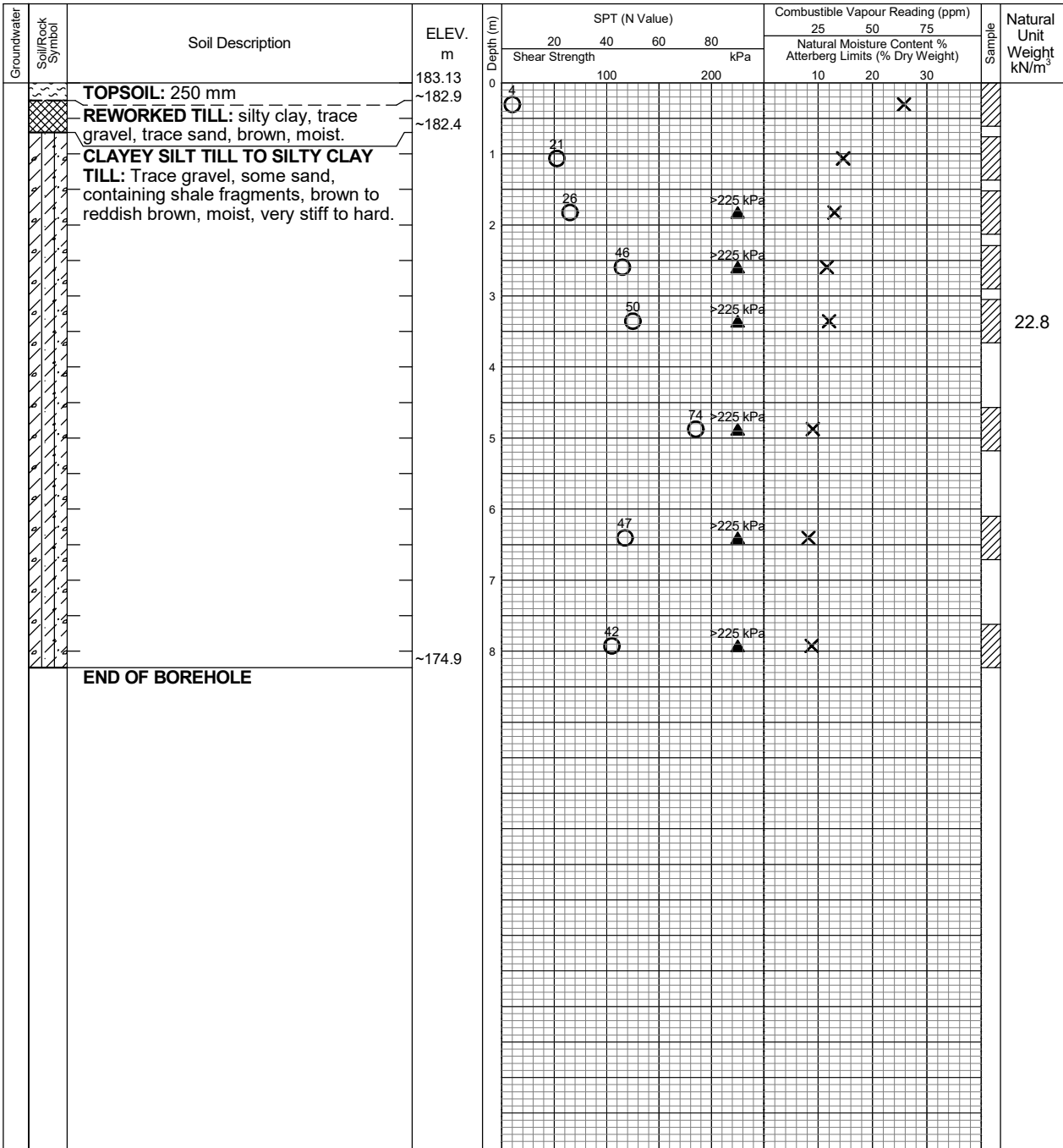
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Datum: Geodetic

EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	Dry	Open

Log of Borehole 1- 4

Project No. BRM-23015226-A0

Drawing No. 5

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 21, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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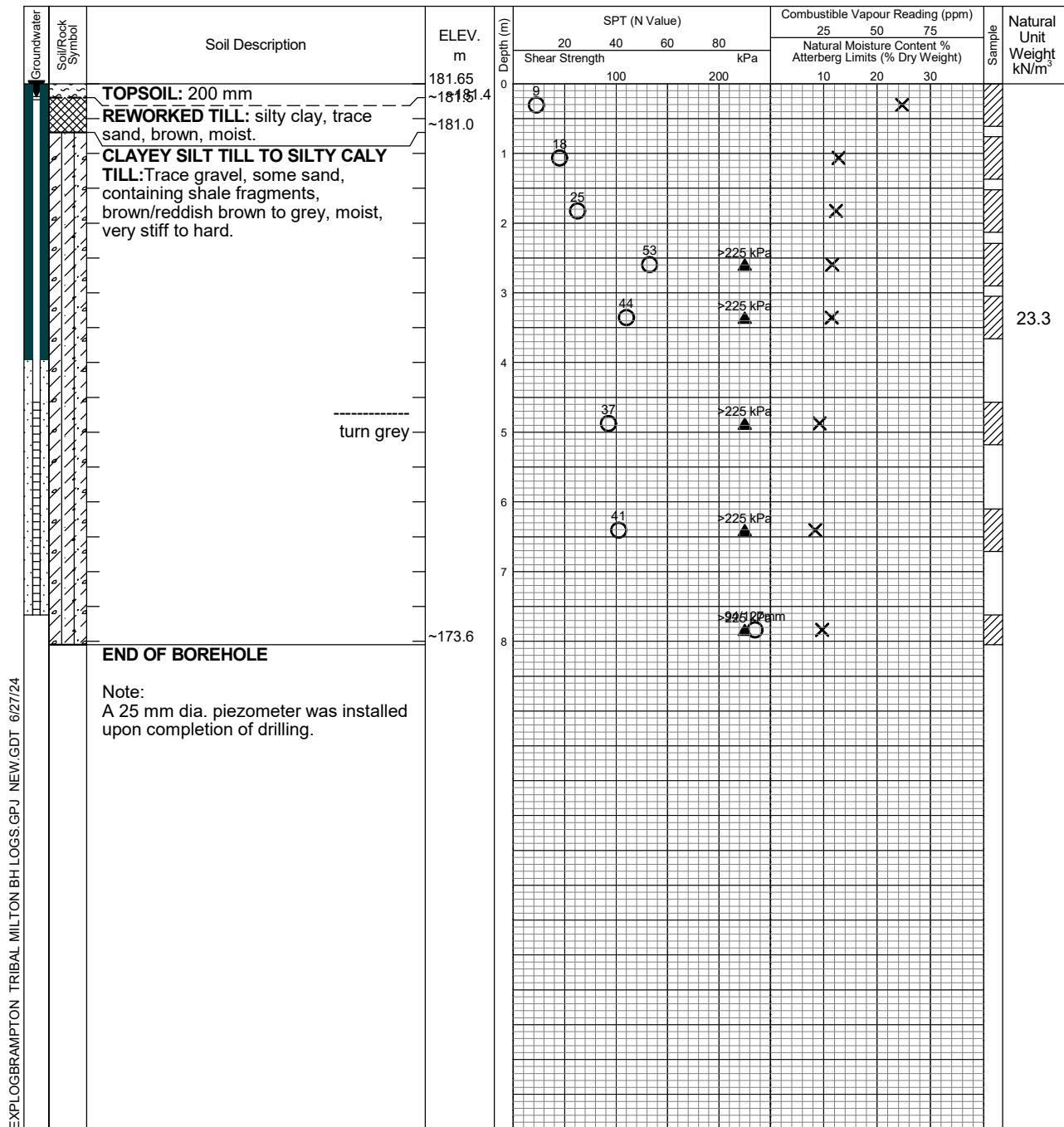
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Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.23	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.37	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.49	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 1.13	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	7.41	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	Dry	Open

Log of Borehole 1-12

Project No. BRM-23015226-A0

Drawing No. 13

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: June 10, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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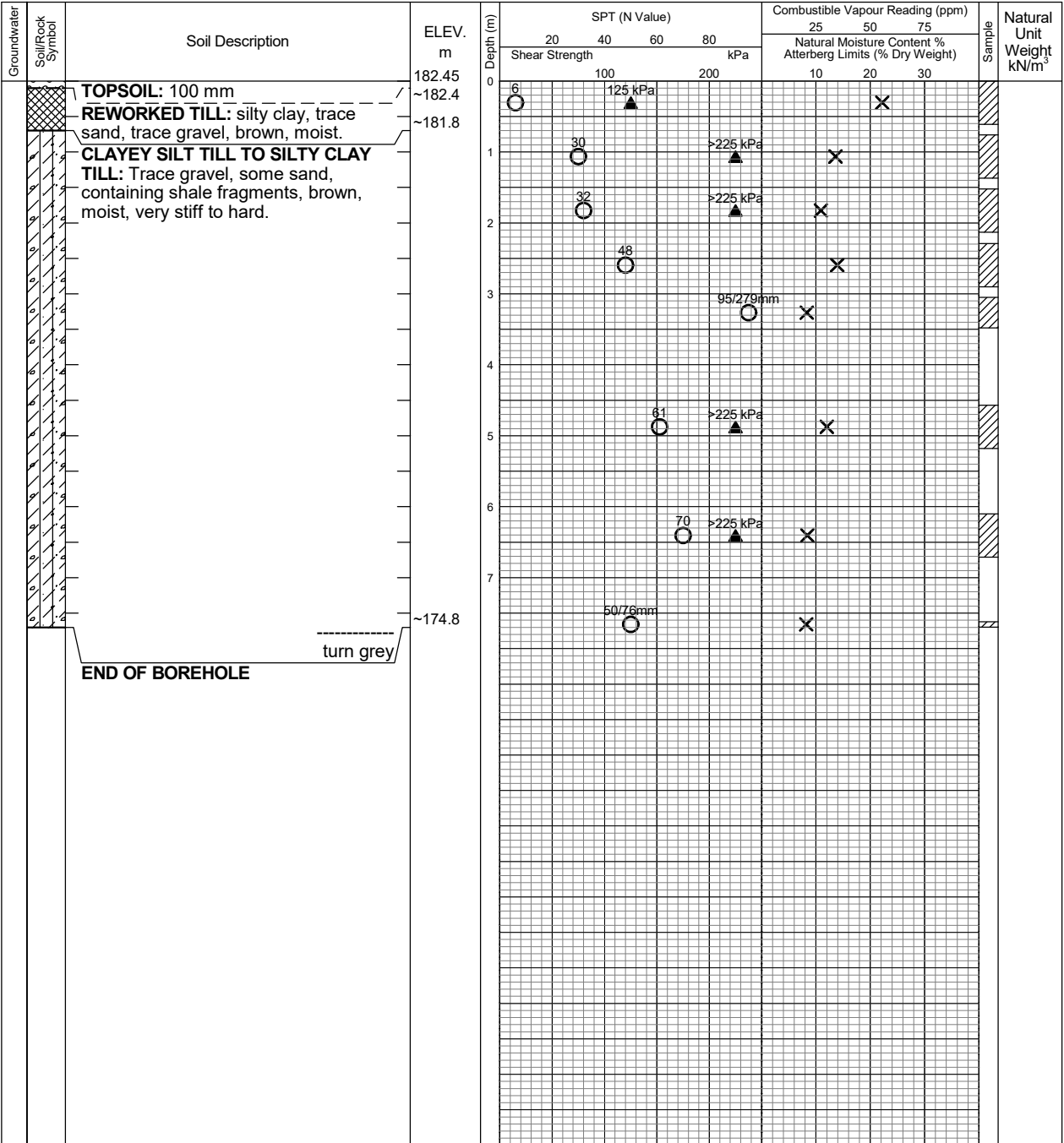
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Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	2.44	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	3.65 2.13	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	Dry	Open

Log of Borehole 1-16

Project No. BRM-23015226-A0

Drawing No. 17

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: June 10, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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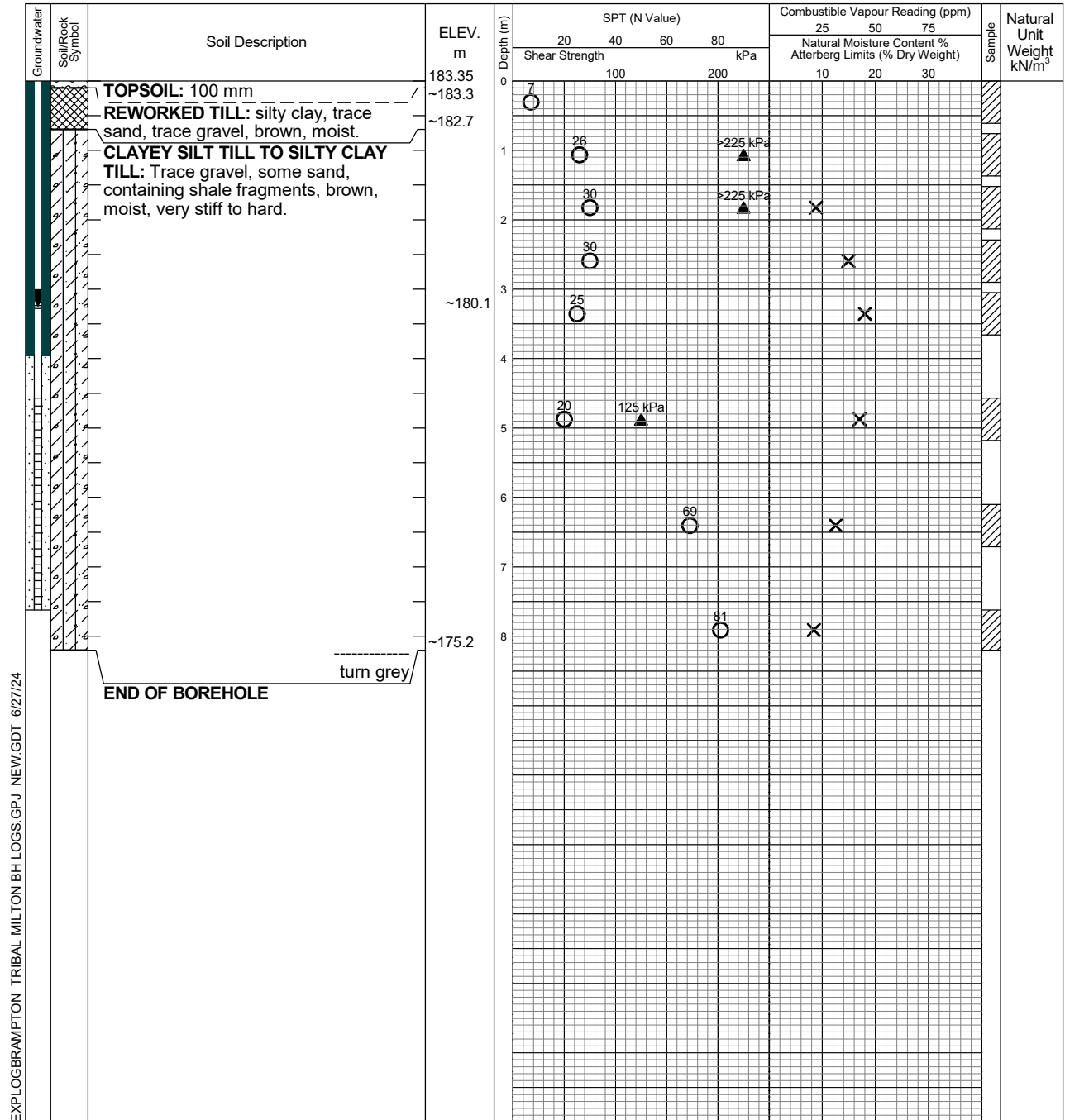
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Datum: Geodetic



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 3.28	Open

Log of Borehole 1-17

Project No. BRM-23015226-A0

Drawing No. 18

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: June 10, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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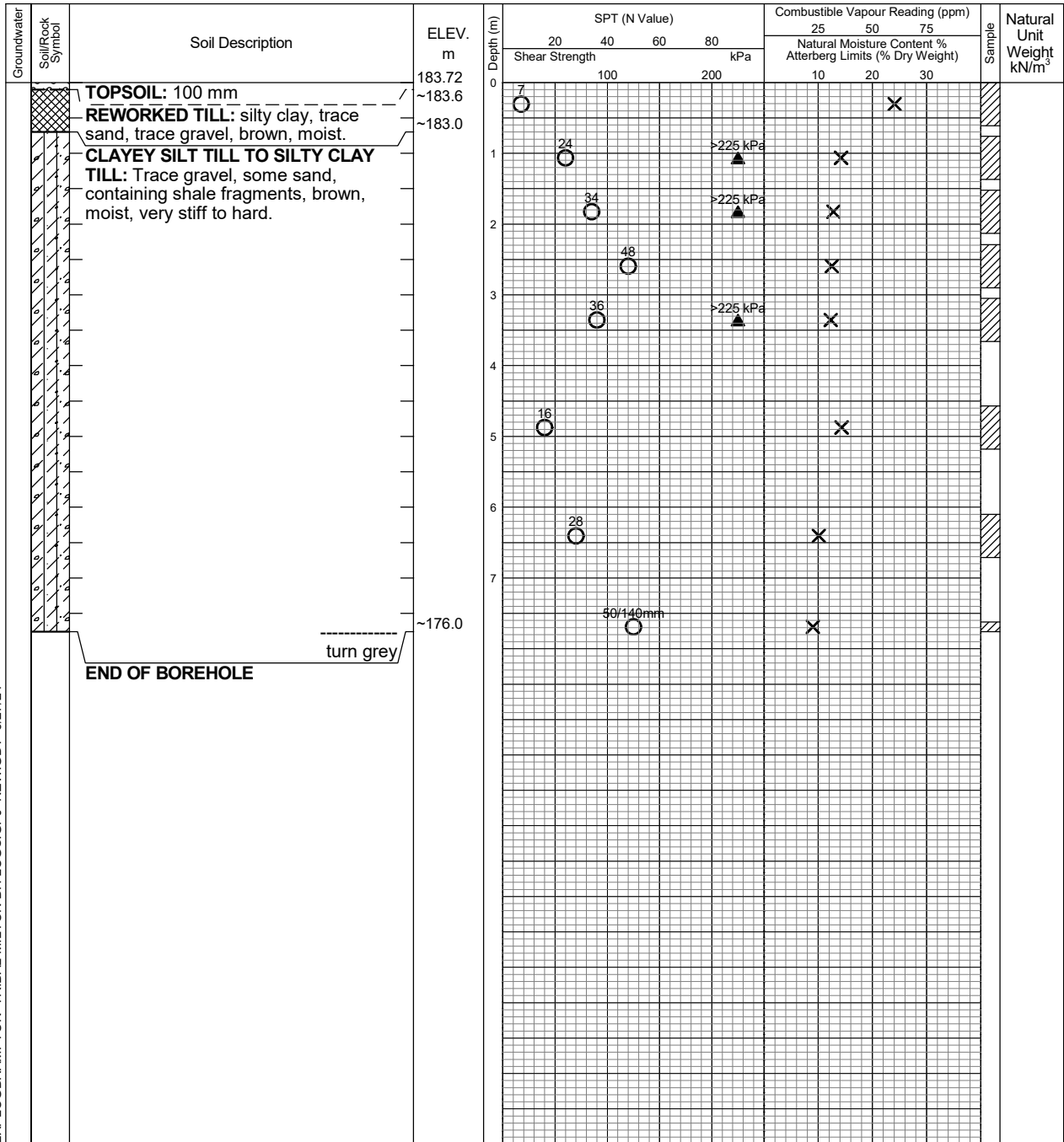
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Datum: Geodetic



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	Dry	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 1.87	Open

Log of Borehole 1-19

Project No. BRM-23015226-A0

Drawing No. 20

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: March 1, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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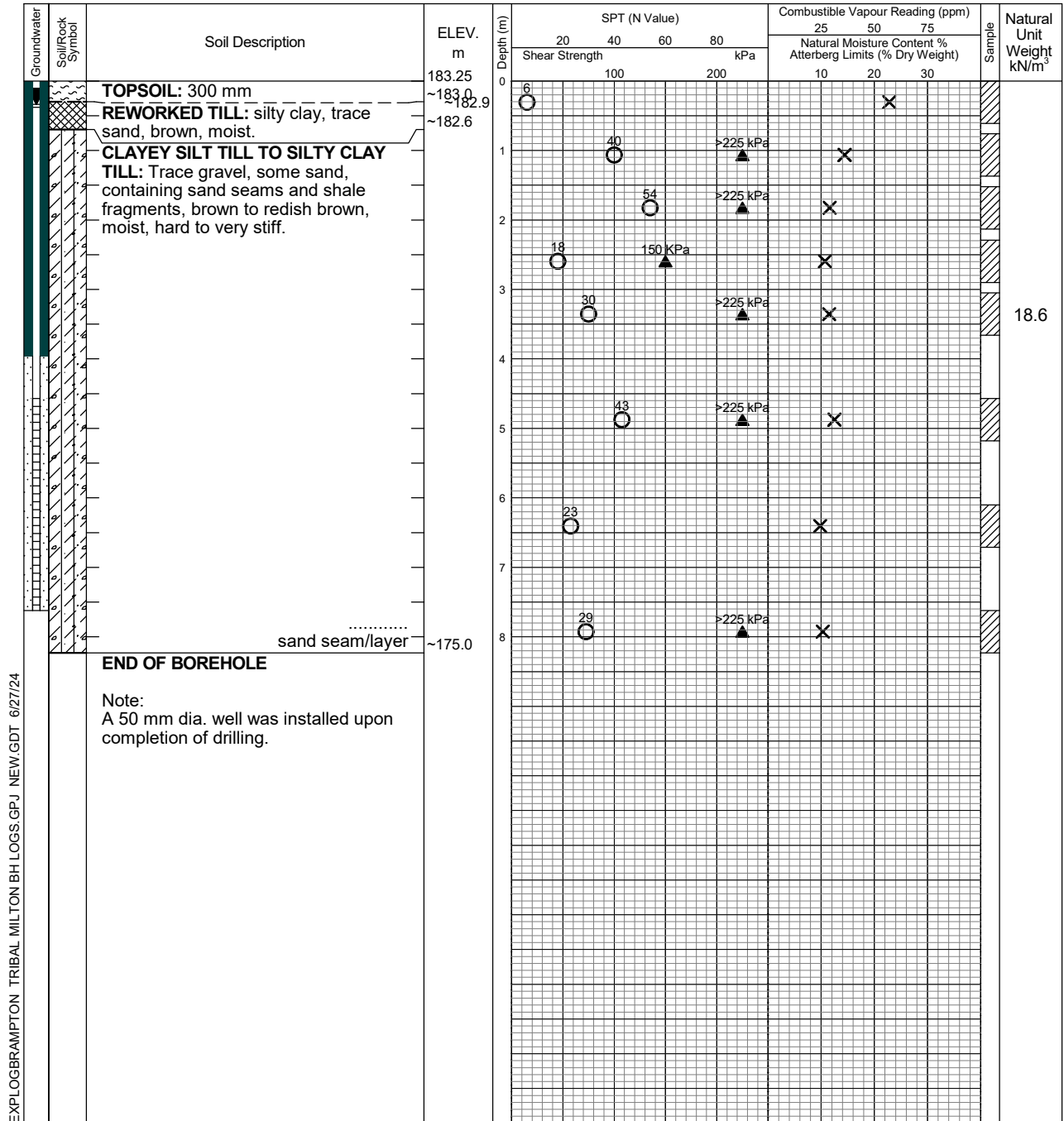
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Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.38	Open

Log of Borehole 1-20

Project No. BRM-23015226-A0

Drawing No. 21

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 29, 2024

Auger Sample

SPT (N) Value

Drill Type: Solid Auger 110mm

Dynamic Cone Test

Shelby Tube

Datum: Geodetic

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

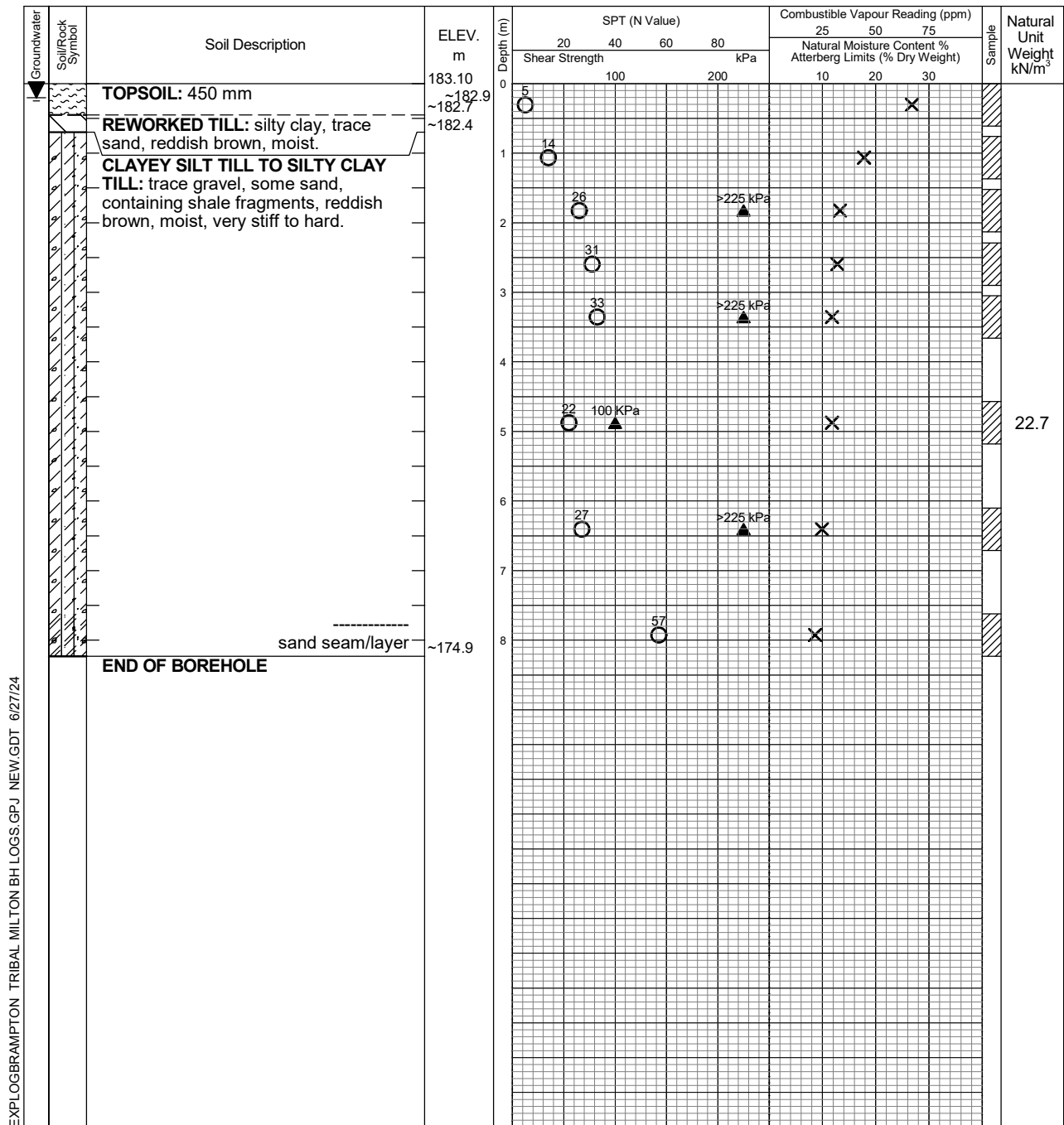
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New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.25	Open

Log of Borehole 1-21

Project No. BRM-23015226-A0

Drawing No. 22

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: March 1, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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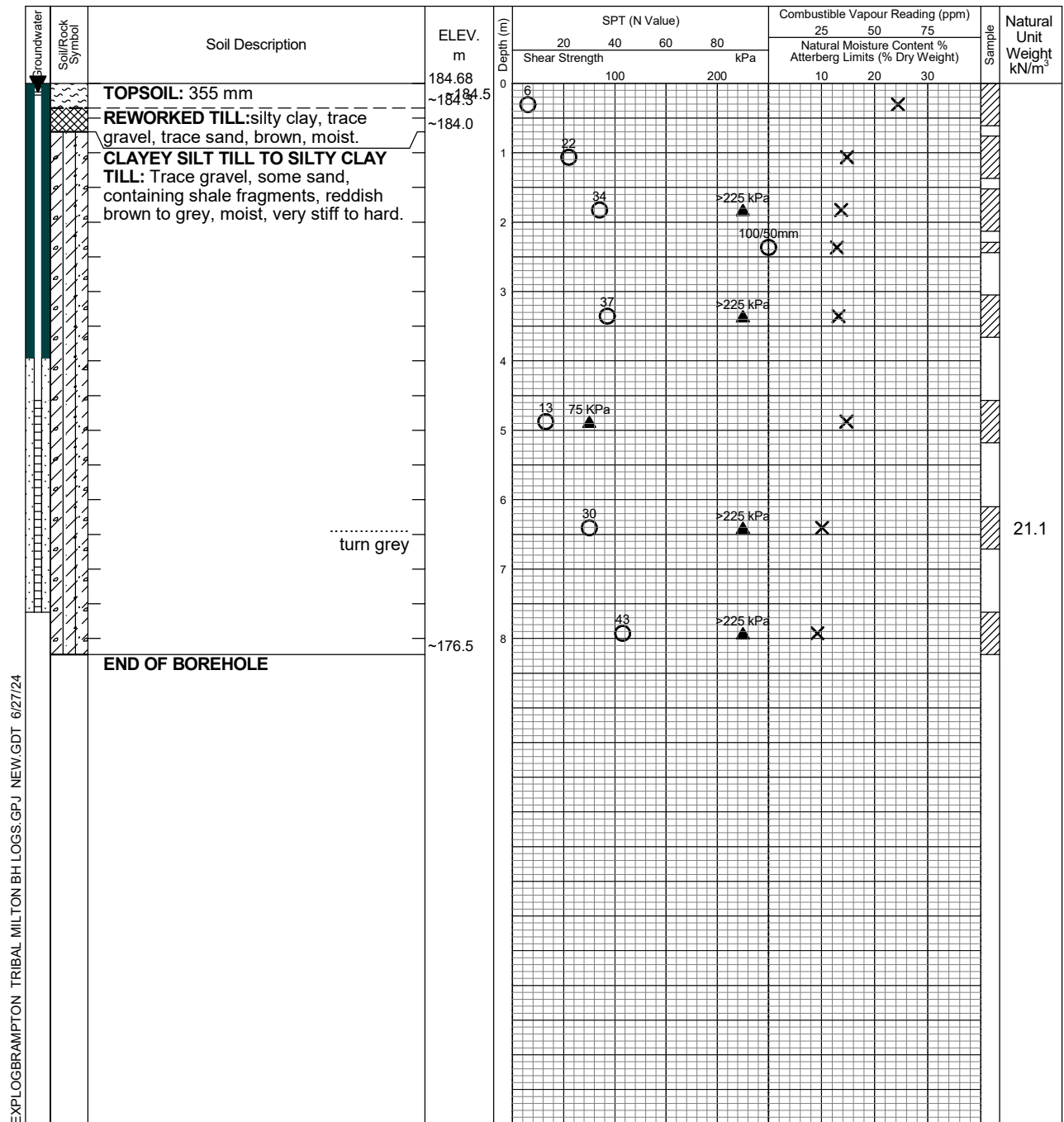
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Datum: Geodetic



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.17	Open

Log of Borehole 1-22

Project No. BRM-23015226-A0

Drawing No. 23

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 29, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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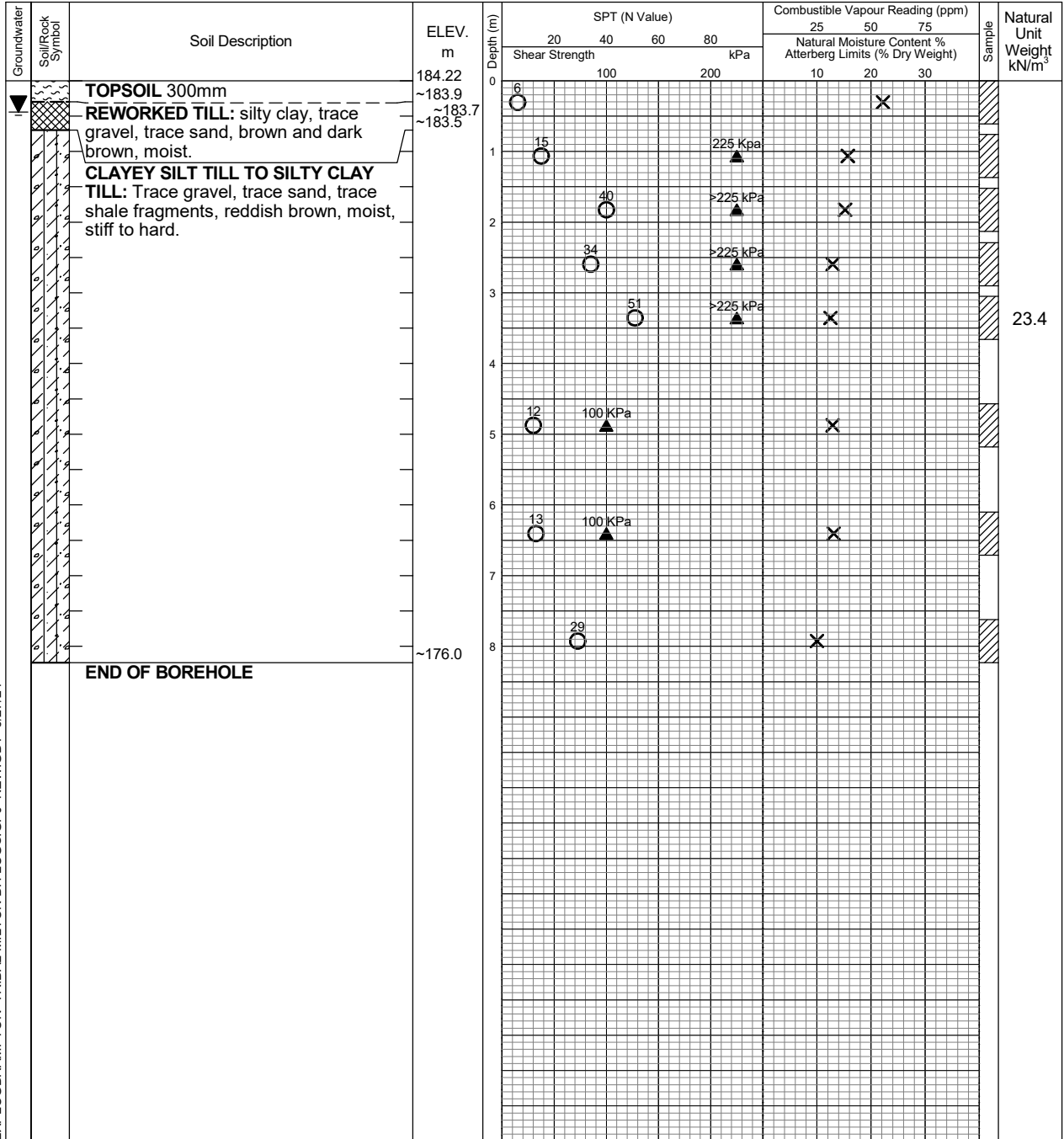
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Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.49	Open

Log of Borehole 1-23

Project No. BRM-23015226-A0

Drawing No. 24

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 27, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

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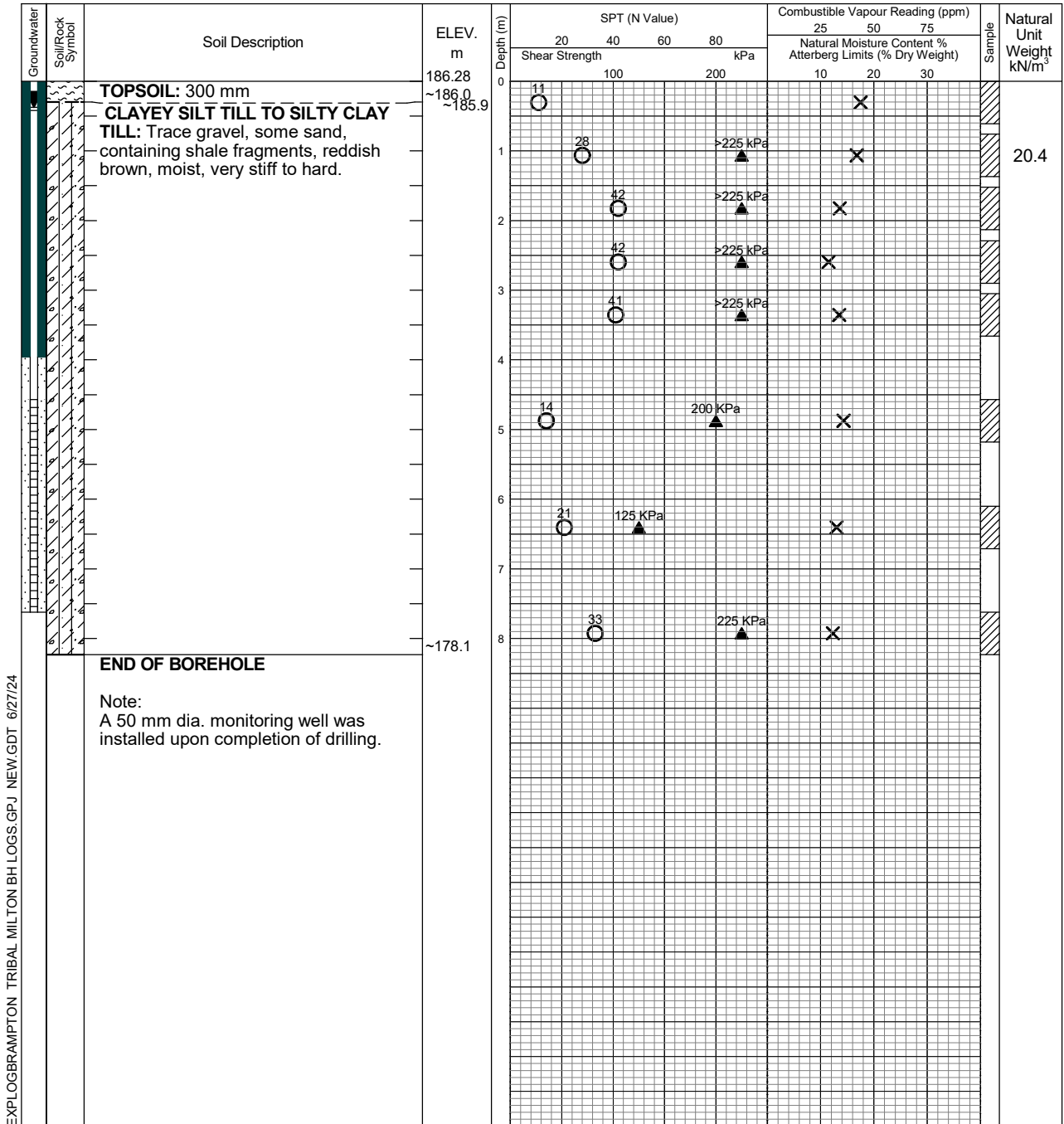
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Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.42	Open

Log of Borehole 1-24

Project No. BRM-23015226-A0

Drawing No. 25

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 29, 2024

Auger Sample

SPT (N) Value

Dynamic Cone Test

Shelby Tube

Field Vane Test

Combustible Vapour Reading

Natural Moisture

Plastic and Liquid Limit

Undrained Triaxial at

% Strain at Failure

Penetrometer

☐

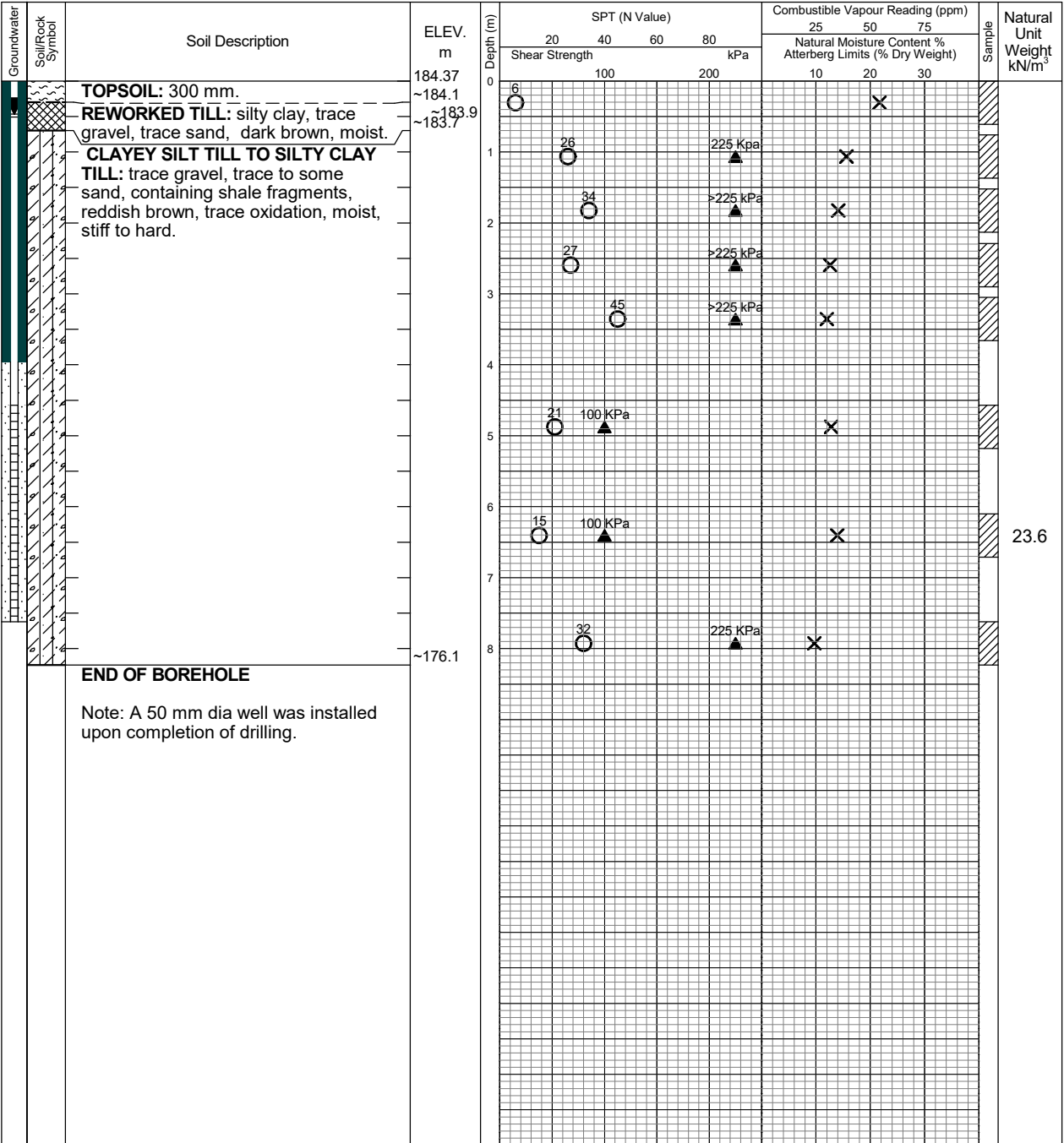
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Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.51	Open

Log of Borehole 1-25

Project No. BRM-23015226-A0

Drawing No. 26

Project: Geotechnical and Hydrogeological Investigation

Sheet No. 1 of 1

Location: Britannia rd and First line ,Milton

See Borehole Location Plan

Date Drilled: February 26, 2024

Auger Sample



SPT (N) Value



Dynamic Cone Test



Shelby Tube



Field Vane Test



Combustible Vapour Reading



Natural Moisture



Plastic and Liquid Limit



Undrained Triaxial at



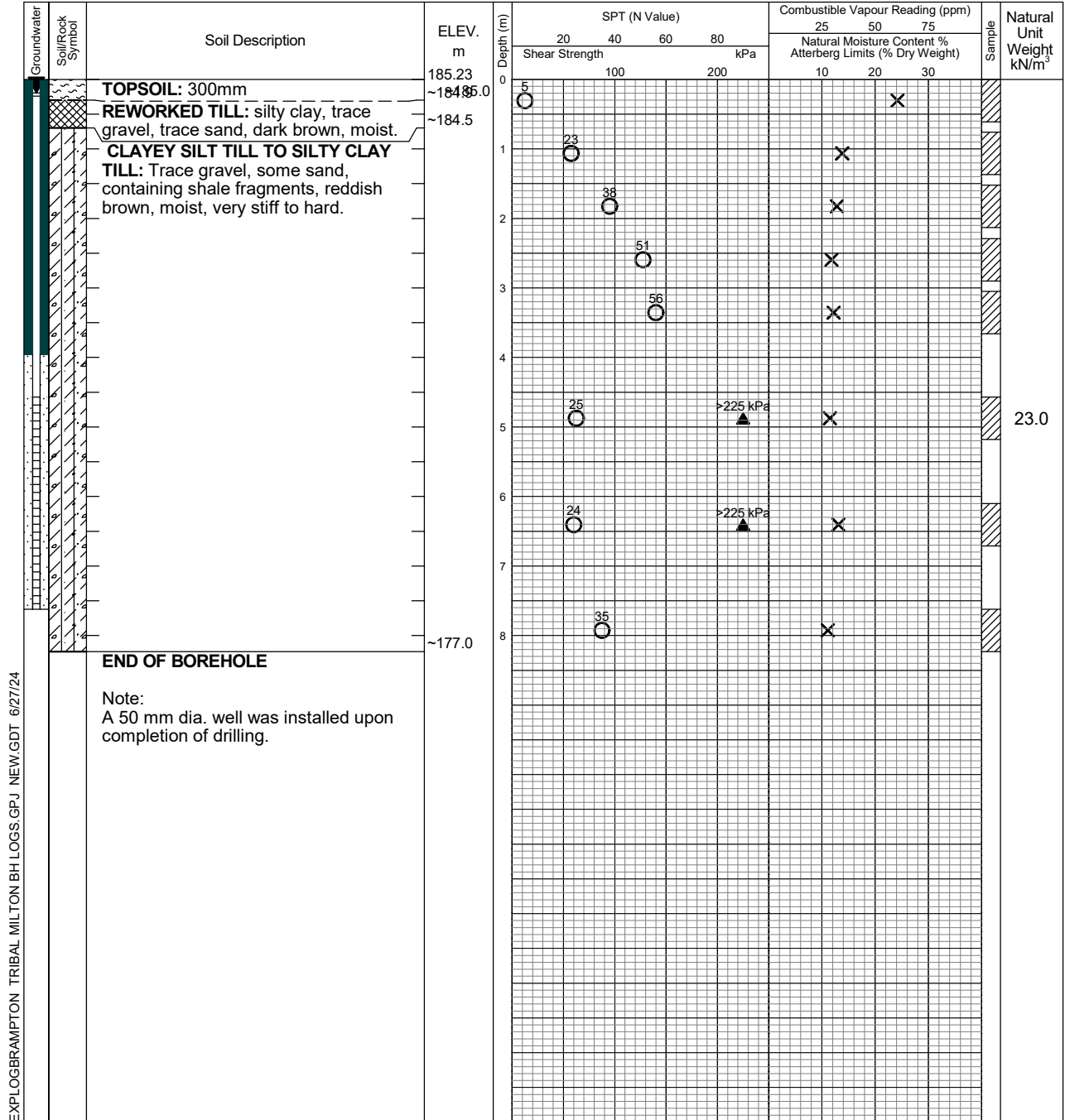
% Strain at Failure



Penetrometer



Datum: Geodetic



EXPLOGBRAMPTON TRIBAL MILTON BH LOGS.GPJ NEW.GDT 6/27/24



New identity of Trow Associates Inc.

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.24	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.80	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.26	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion June 26, 2024	Dry 0.90	Open

Elapsed Time	Water Level (m)	Hole Open to (m)
Upon Completion	Dry	Open

Appendix A:

Results of Grain Size Analyses



exp Services Inc.
1595 Clark Boulevard, Brampton
Ontario, Canada, L6T 4V1
Telephone: (905) 793-9800
Fax: (905) 793-0641

Grain Size Analysis & Hydrometer Test Report

ST08

Sample Test No.: 441977-3

Report No.: 1

Date Reported: 27-Feb-24

Project No.: brm-23015226-a0 101

Project Name: Geo Phase 1

Grain Size Proportion (%)

Gravel (> 4.75mm): 4.7
Sand (> 75µm, < 4.75mm): 18.8
Silt (> 2µm, < 75µm): 54.8
Clay (< 2µm): 21.7
Total: 100.0

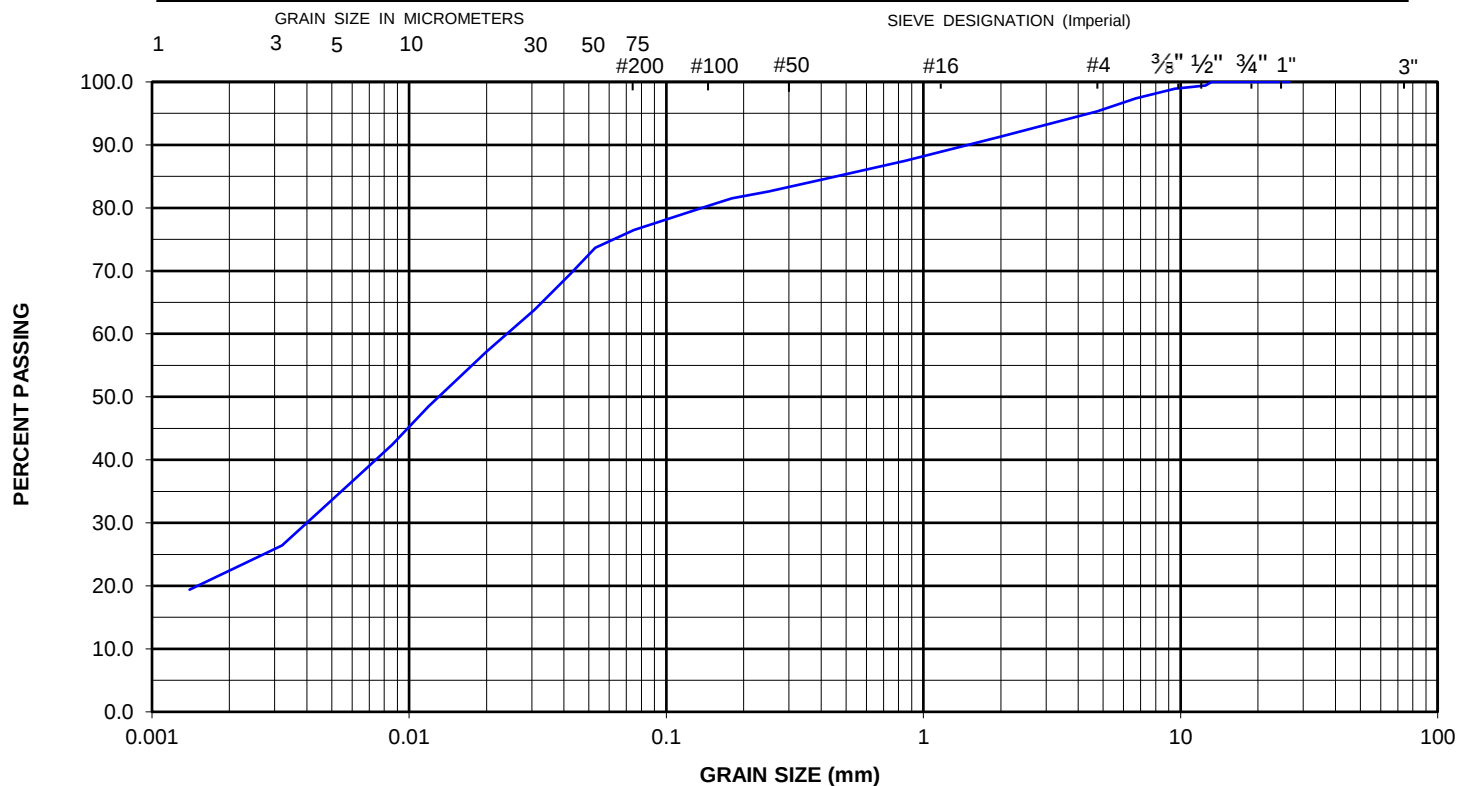
Sample Information

Location: BH 1-9
Sample Method: SS
Sample No.: 3
Depth: 1.5 - 2.1 m
Sample Description: Silty Clay Till, some Sand, trace Gravel; Brown
Sampled By: G.S.
Sampling Date: 2/22/2024
Date Received: 2/22/2024
Client Sample ID:
Comments:

Grain Size (mm)	% Passing	Grain Size (mm)	% Passing
26.5	100.0	0.0424	69.5
22.4	100.0	0.0307	63.8
19	100.0	0.0200	57.2
16	100.0	0.0119	48.5
13.2	100.0	0.0086	42.4
12.5	99.4	0.0062	37.1
9.5	98.9	0.0032	26.4
6.7	97.4	0.0014	19.4
4.75	95.3		
2	91.3		
0.85	87.5		
0.425	84.7		
0.25	82.6		
0.18	81.5		
0.15	80.5		
0.075	76.5		
0.053	73.7		

UNIFIED SOIL CLASSIFICATION SYSTEM

CLAY AND SILT	SAND			GRAVEL	
	Fine	Medium	Coarse	Fine	Coarse



Project Manager: Yuanpeng Li

Approved By: Original Signed By
Arcadio Petrola, Lab Supervisor

Date Approved: 27-Feb-24



exp Services Inc.
1595 Clark Boulevard, Brampton
Ontario, Canada, L6T 4V1
Telephone: (905) 793-9800
Fax: (905) 793-0641

Plasticity Index Test Report

ST03

Project No.: Brm-23015226-A0

Sample Number: 441977-4

Date Sampled: February 22, 2024

Date Received: February 22, 2024

Date Reported: February 28, 2024

Borehole No: BH 1-9 / SS3

Sample Depth: 1.5 - 2.1 m

Liquid Limit

Trial Number	1	2	3	4	5
Number of Blows	39	24	14		
Moisture Tin ID	A	B	C		
Mass of Soil and Tin, g	19.565	17.791	18.282		
Mass of Dry Soil and Tin, g	15.758	14.208	14.366		
Mass of Tin, g	1.738	1.761	1.782		
Mass of Water, g	3.807	3.583	3.916		
Mass of Dry Soil, g	14.020	12.447	12.584		
Water Content	27.2%	28.8%	31.1%		

Plastic Limit

Trial Number	1	2	3
Moisture Tin ID	V	W	X
Mass of Soil and Tin, g	17.733	18.427	18.870
Mass of Dry Soil and Tin, g	15.725	16.346	16.808
Mass of Tin, g	1.726	1.750	1.756
Mass of Water, g	2.008	2.081	2.062
Mass of Dry Soil, g	13.999	14.596	15.052
Water Content	14.3%	14.3%	13.7%

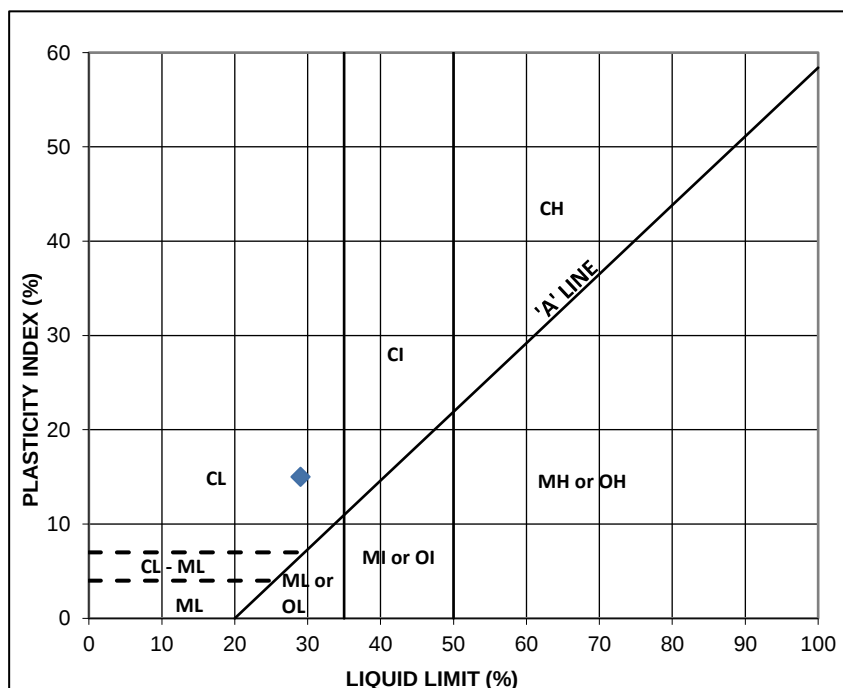
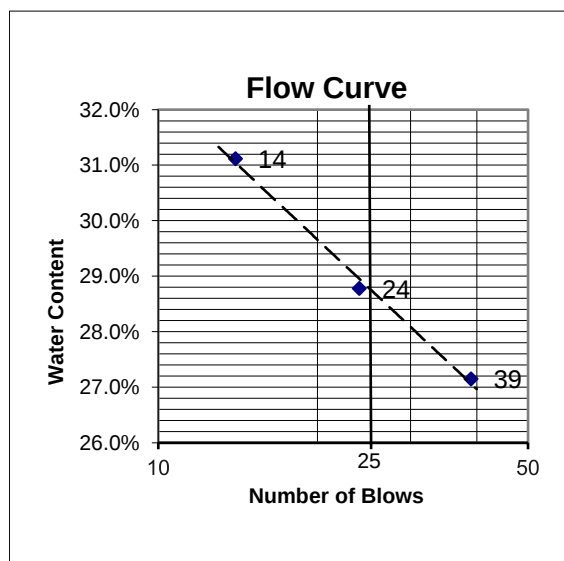
Summary of Results

Liquid Limit (LL) : 29

Plastic Limit (PL) : 14

Plasticity Index (PI) : 15

Classification: **CL**



Tested By: **Carlito Picache, C.Tech.**
Lab. Technician

Checked By: **Arcadio Petrola, CET**
Senior Lab. Technician



exp Services Inc.
1595 Clark Boulevard, Brampton
Ontario, Canada, L6T 4V1
Telephone: (905) 793-9800
Fax: (905) 793-0641

Grain Size Analysis & Hydrometer Test Report

ST08

Sample Test No.: 442182-2

Report No.: 2

Date Reported: 01-Mar-24

Project No.: brm-23015226-a0 101

Project Name: Geo Phase 1

Grain Size Proportion (%)

Gravel (> 4.75mm): 3.1
Sand (> 75µm, < 4.75mm): 17.0
Silt (> 2µm, < 75µm): 53.7
Clay (< 2µm): 26.2
Total: 100.0

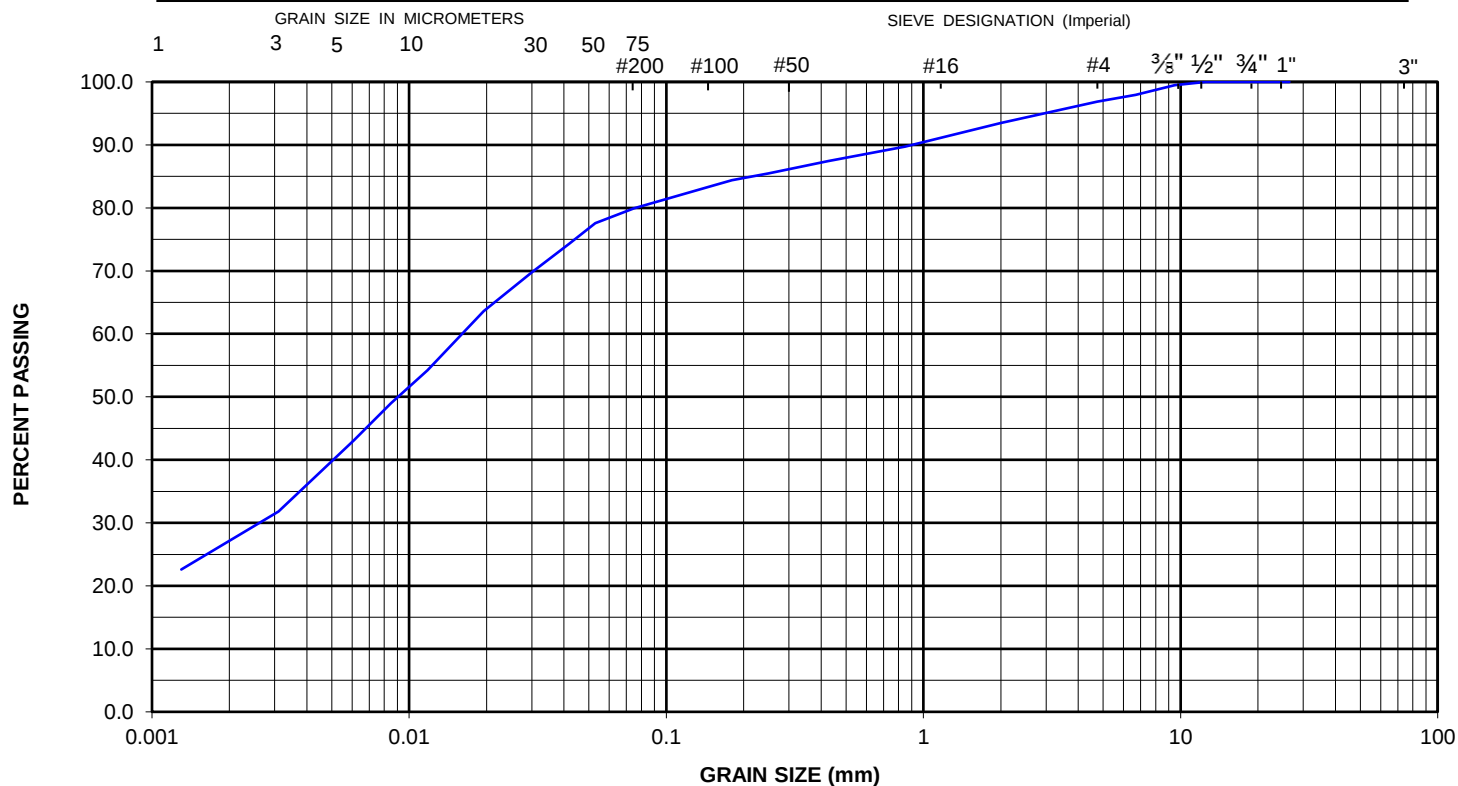
Sample Information

Location: BH 1-25
Sample Method: SS
Sample No.: 3
Depth: 1.5 - 2.1 m
Sample Description: Silty Clay Till, some Sand, trace Gravel; Brown
Sampled By: G.S.
Sampling Date: 2/26/2024
Date Received: 2/26/2024
Client Sample ID:
Comments:

Grain Size (mm)	% Passing	Grain Size (mm)	% Passing
26.5	100.0	0.0419	74.3
22.4	100.0	0.0301	69.8
19	100.0	0.0195	63.6
16	100.0	0.0118	54.2
13.2	100.0	0.0085	49.0
12.5	100.0	0.0061	43.1
9.5	99.5	0.0031	31.8
6.7	98.0	0.0013	22.6
4.75	96.9		
2	93.5		
0.85	89.7		
0.425	87.4		
0.25	85.5		
0.18	84.4		
0.15	83.5		
0.075	79.9		
0.053	77.6		

UNIFIED SOIL CLASSIFICATION SYSTEM

CLAY AND SILT	SAND			GRAVEL	
	Fine	Medium	Coarse	Fine	Coarse



Project Manager: Yuanpeng Li

Approved By: Original Signed By
Arcadio Petrola, Lab Supervisor

Date Approved: 01-Mar-24

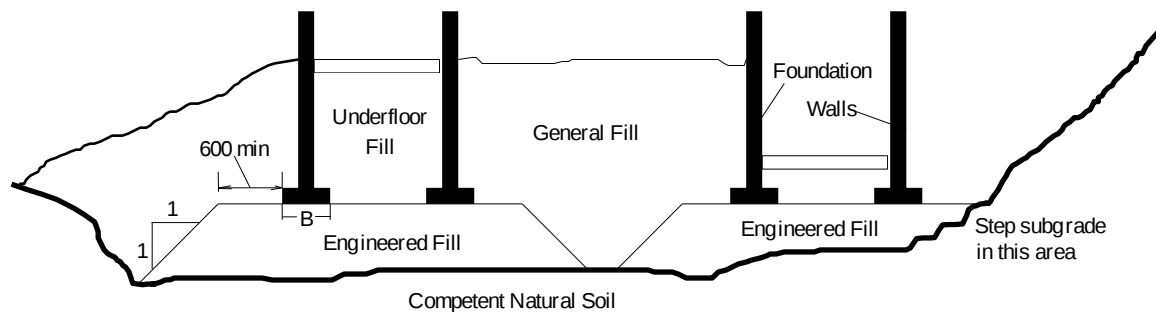
Appendix B:

Engineered Fill Certificates

Foundations placed on engineered fill comprising native soil from the site - or imported materials - may be designed for an SLS geotechnical reaction of 150 kPa (ULS factored geotechnical resistance of 225 kPa).

Additional comments with regard to engineered fill are as follows:

- The area must be stripped of all topsoil, existing fill material or other deleterious material and proof-rolled. Soft spots must be dug out. The stripped native subgrade must be examined and approved by a geotechnical engineer prior to placement of fill.
- The approved engineered fill must be placed in loose lifts not exceeding 200 mm and compacted to 100% Standard Proctor dry density throughout. Granular fill is preferred.
- Full time geotechnical inspection during placement of engineered fill is required.
- The fill must be placed such that the specified geometry is achieved as follows:.



Foundations on Engineered Fill (schematic)

- A minimum footing width of 500 mm (20 inches) is suggested. It is recommended that poured concrete basement walls should be reinforced with 2 – 15M rebars above the footings and below the window sill.
- All excavations must be done in accordance with the Occupational Health and Safety Regulations of Ontario.

Appendix C: Certificates of Analyses



Attention: Jack Li

exp Services Inc
Brampton Branch
1595 Clark Blvd
Brampton, ON
CANADA L6T 4V1

Your P.O. #: BRAMPTON GEOTECHNICA
Your Project #: BRM-23015226-A0/101
Site Location: BRITANNIA ROAD AND FIRST LINE, MILTON
Your C.O.C. #: N/A

Report Date: 2024/06/25
Report #: R8207528
Version: 1 - Final

CERTIFICATE OF ANALYSIS

BUREAU VERITAS JOB #: C416759

Received: 2024/06/19, 11:55

Sample Matrix: Soil
Samples Received: 1

Analyses	Quantity	Date	Date	Laboratory Method	Analytical Method
		Extracted	Analyzed		
pH CaCl2 EXTRACT	1	2024/06/24	2024/06/24	CAM SOP-00413	EPA 9045 D m
Sulphate (20:1 Extract)	1	2024/06/24	2024/06/25	CAM SOP-00464	MOE E3013 m

Remarks:

Bureau Veritas is accredited to ISO/IEC 17025 for specific parameters on scopes of accreditation. Unless otherwise noted, procedures used by Bureau Veritas are based upon recognized Provincial, Federal or US method compendia such as CCME, EPA, APHA or the Quebec Ministry of Environment.

All work recorded herein has been done in accordance with procedures and practices ordinarily exercised by professionals in Bureau Veritas' profession using accepted testing methodologies, quality assurance and quality control procedures (except where otherwise agreed by the client and Bureau Veritas in writing). All data is in statistical control and has met quality control and method performance criteria unless otherwise noted. All method blanks are reported; unless indicated otherwise, associated sample data are not blank corrected. Where applicable, unless otherwise noted, Measurement Uncertainty has not been accounted for when stating conformity to the referenced standard.

Bureau Veritas liability is limited to the actual cost of the requested analyses, unless otherwise agreed in writing. There is no other warranty expressed or implied. Bureau Veritas has been retained to provide analysis of samples provided by the Client using the testing methodology referenced in this report. Interpretation and use of test results are the sole responsibility of the Client and are not within the scope of services provided by Bureau Veritas, unless otherwise agreed in writing. Bureau Veritas is not responsible for the accuracy or any data impacts, that result from the information provided by the customer or their agent.

Solid sample results, except biota, are based on dry weight unless otherwise indicated. Organic analyses are not recovery corrected except for isotope dilution methods.

Results relate to samples tested. When sampling is not conducted by Bureau Veritas, results relate to the supplied samples tested.

This Certificate shall not be reproduced except in full, without the written approval of the laboratory.

Reference Method suffix "m" indicates test methods incorporate validated modifications from specific reference methods to improve performance.

* RPDs calculated using raw data. The rounding of final results may result in the apparent difference.



Attention: Jack Li

exp Services Inc
Brampton Branch
1595 Clark Blvd
Brampton, ON
CANADA L6T 4V1

Your P.O. #: BRAMPTON GEOTECHNICA
Your Project #: BRM-23015226-A0/101
Site Location: BRITANNIA ROAD AND FIRST LINE, MILTON
Your C.O.C. #: N/A

Report Date: 2024/06/25
Report #: R8207528
Version: 1 - Final

CERTIFICATE OF ANALYSIS

BUREAU VERITAS JOB #: C416759

Received: 2024/06/19, 11:55

Encryption Key

Please direct all questions regarding this Certificate of Analysis to:

Patricia Legette, Project Manager

Email: Patricia.Legette@bureauveritas.com

Phone# (905)817-5799

=====

This report has been generated and distributed using a secure automated process.

Bureau Veritas has procedures in place to guard against improper use of the electronic signature and have the required "signatories", as per ISO/IEC 17025, signing the reports. For Service Group specific validation, please refer to the Validation Signatures page if included, otherwise available by request. For Department specific Analyst/Supervisor validation names, please refer to the Test Summary section if included, otherwise available by request. This report is authorized by Rodney Major, General Manager responsible for Ontario Environmental laboratory operations.



RESULTS OF ANALYSES OF SOIL

Bureau Veritas ID		ZMU803		
Sampling Date		2024/06/10 15:00		
COC Number		N/A		
	UNITS	BOREHOLE 1-16 - SS3	RDL	QC Batch
Inorganics				
Available (CaCl2) pH	pH	7.92		9474426
Soluble (20:1) Sulphate (SO4)	ug/g	960	20	9474077
RDL = Reportable Detection Limit				
QC Batch = Quality Control Batch				



Bureau Veritas Job #: C4I6759
Report Date: 2024/06/25

exp Services Inc
Client Project #: BRM-23015226-A0/101
Site Location: BRITANNIA ROAD AND FIRST LINE, MILTON
Your P.O. #: BRAMPTON GEOTECHNICA
Sampler Initials: JL

TEST SUMMARY

Bureau Veritas ID: ZMU803
Sample ID: BOREHOLE 1-16 - SS3
Matrix: Soil

Collected: 2024/06/10
Shipped:
Received: 2024/06/19

Test Description	Instrumentation	Batch	Extracted	Date Analyzed	Analyst
pH CaCl2 EXTRACT	AT	9474426	2024/06/24	2024/06/24	Kien Tran
Sulphate (20:1 Extract)	SKAL/EC	9474077	2024/06/24	2024/06/25	Alina Dobreanu



BUREAU
VERITAS

Bureau Veritas Job #: C4I6759
Report Date: 2024/06/25

exp Services Inc
Client Project #: BRM-23015226-A0/101
Site Location: BRITANNIA ROAD AND FIRST LINE, MILTON
Your P.O. #: BRAMPTON GEOTECHNICA
Sampler Initials: JL

GENERAL COMMENTS

Each temperature is the average of up to three cooler temperatures taken at receipt

Package 1	0.0°C
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Results relate only to the items tested.



BUREAU
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Bureau Veritas Job #: C4I6759

Report Date: 2024/06/25

QUALITY ASSURANCE REPORT

exp Services Inc

Client Project #: BRM-23015226-A0/101

Site Location: BRITANNIA ROAD AND FIRST LINE, MILTON

Your P.O. #: BRAMPTON GEOTECHNICA

Sampler Initials: JL

QC Batch	Parameter	Date	Matrix Spike		SPIKED BLANK		Method Blank		RPD	
			% Recovery	QC Limits	% Recovery	QC Limits	Value	UNITS	Value (%)	QC Limits
9474077	Soluble (20:1) Sulphate (SO4)	2024/06/25	NC	70 - 130	104	70 - 130	<20	ug/g	19	35
9474426	Available (CaCl2) pH	2024/06/24			100	97 - 103			0.53	N/A

N/A = Not Applicable

Duplicate: Paired analysis of a separate portion of the same sample. Used to evaluate the variance in the measurement.

Matrix Spike: A sample to which a known amount of the analyte of interest has been added. Used to evaluate sample matrix interference.

Spiked Blank: A blank matrix sample to which a known amount of the analyte, usually from a second source, has been added. Used to evaluate method accuracy.

Method Blank: A blank matrix containing all reagents used in the analytical procedure. Used to identify laboratory contamination.

NC (Matrix Spike): The recovery in the matrix spike was not calculated. The relative difference between the concentration in the parent sample and the spike amount was too small to permit a reliable recovery calculation (matrix spike concentration was less than the native sample concentration)



BUREAU
VERITAS

Bureau Veritas Job #: C4I6759

Report Date: 2024/06/25

exp Services Inc

Client Project #: BRM-23015226-A0/101

Site Location: BRITANNIA ROAD AND FIRST LINE, MILTON

Your P.O. #: BRAMPTON GEOTECHNICA

Sampler Initials: JL

VALIDATION SIGNATURE PAGE

The analytical data and all QC contained in this report were reviewed and validated by:

Cristina Carriere, Senior Scientific Specialist

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CAM FCD-01191/6